#### STORMWATER MANAGEMENT AND CONSERVATION

by
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#### Introduction

This paper reports on a preliminary analysis of a hydrologic balance within the Econlockhatchee River Basin. The work was completed to indicate the need for conservation of our water resources within the river basin. Specifically, the analysis documents increasing runoff from urban developed areas relative to rural areas.

Once urbanization takes place without regard to conservation of an existing hydrologic balance, water can be lost to our rivers and oceans, flooding potential increases, groundwaters are depleted, and pollution of the waters occurs. These consequences have been documented using a hydrologic balance for other areas within the State of Florida (Wanielista, 1990). The hydrologic cycle can be used to document some consequences resulting from urban development in the Econlockhatchee River.

Common sense would dictate that water falling on impervious surfaces that is not diverted to pervious surfaces will run off to streams and rivers. This runoff water is lost from the impervious surfaces of urban lands to the Econlockhatchee River. However, not all the runoff water has to be discharged, part of it can be reused within the watershed as a source of water for non-potable uses.

#### River Flow

Using the last 10 years of streamflow data in the River as reported by the U.S.G.S. at the Union Park and the Magnolia Ranch gaging sites, the streamflows are compared. The Magnolia Ranch station records data from the rural environment, while the Union Park site represents an urban area. The data are reported in units of inches over each square mile per year and compared to rainfall volumes as shown in Table 1. The rural area river flow averaged about 23% of the rainfall while the urban area river flow averaged about 33% of the rainfall and was approximately 45% greater than the rural.

TABLE 1

Hydrologic Balance Comparisons for Two Sites on the Econlockhatchee River
. (using 1979-1988 Data)\*

	Urban	Rural	
Precipitation (inches/year) (at Orlando International, adjusted by data from U.C.F.)	49	49	
Riverflow (inches/year) Evapotranspiration/Recharge (inches/year)	16 33	11 38	

\*Data rounded to nearest inch

The contribution of the urban areas also can be documented by showing graphically the cumulative amount of water being discharged in the river at both sites (Figure 1). Over a ten year period, the quantity of discharged water per square mile at the urban site is greater than the quantity from the rural site. The difference is

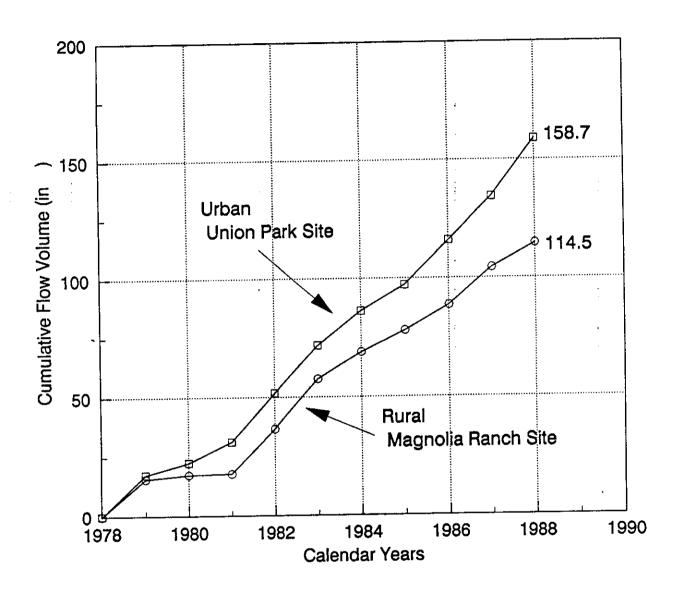


Figure 1. Cumulative Flow Volume Comparison / Econlockhatchee River

about 44 inches which is almost equal to the average yearly rainfall. About every 10-11 years based on recent data, we are loosing about one year of rainfall. The loss of water from the 27.1 square mile urban watershed is equivalent to the water used by 570,000 people at a rate of 100 gallons per day for one full year.

Also, there is an increasing "gap" or divergence between the flows from the urban and rural sites as time increases due presumably to urbanization activities. We are loosing water and increasing pollution loads to the river. Also the potential for flooding is increasing. This increased flow is directly attributed to the impervious surface of the urban areas. Rainfall on these surfaces that are hydraulically connected to the river will be discharged to the river unless stormwater management to reuse the stormwater is practiced.

#### Stormwater Management and Reuse

Extensive research with practiced applications are underway at the University of Central Florida to help design and operate stormwater reuse systems. The objectives of the designs are to reduce both pollution and the volume of water in stormwater discharges. This can be accomplished by reusing the stormwater for car washing, surficial aquifer recharge, cooling water make-up, ornamental foliage, and irrigation.

Irrigation systems have been used before in the local area. We have documented 12 sites in the urban area, but no specific design and operational criteria have been acceptable to the

scientific, engineering and regulatory communities. Other irrigation sites are being used in the rural environment, primarily for agricultural purposes. In other areas of the State, design and operation criteria have been developed, for example in Tallahassee and Manatee County.

The benefits of reuse are both environmentally acceptable and are economical. If irrigation with potable water is being done at 2 inches per week over 100 acres (golf course size), and the potable water cost \$1.00/1000 gallons, the annual cost is over \$300,000 for the water alone. The stormwater is free. The only additional cost for both systems is electricity and the irrigation system. With stormwater, the pumping cost and the pumps would cost about \$30,000/year.

#### Conclusions

The Econlockhatchee River flow is increasing at a greater rate every year. Runoff from urban areas must be controlled and reused to conserve our water resources. There are currently available design criteria for stormwater management ponds that are being used by the technical and regulatory community. These criteria and other environmental factors should be given serious consideration if we are to maintain reasonable hydrologic and economic conditions within the River. The Saint Johns River Water Management District has permitted a few reuse systems and the Florida Department of Environmental Regulation is currently promoting their use as conservation and mitigation methods. It is highly recommended that

stormwater reuse systems be implemented in the Econlockhatchee River Basin. The time is now!

#### Acknowledgements

Some of the technical and financial support for this work was provided by the Florida Department of Environmental Regulation, the Environmental Protection Agency, and the Saint Johns River Water Management District. It is a continuing effort and the help of individuals within these agencies is appreciated.

#### References

Wanielista, M. P., 1990, <u>Evers Reservoir Hydrologic Balance</u>,

University of Central Florida, Orlando, Fl.

Wanielista, M. P., 1990a, <u>Hydrology and Water Ouantity Control</u>, J. Wiley and Sons, New York, N. Y.

### Discharge, in cubic feet per second.

Magnolia Rai	nch(32.9 m <sup>2</sup> )	Union Park (27.1m <sup>2</sup> )						
Water Year	Mean(cfr/yn)	Water Year	₩ean (cfr/yr) 48.7					
1987	38	1987	48.7					
1988	24.8	1988	37.4					
1989	26.3	1989	21.3					
1990	5.01	1990	13.4					
1991	43.5	1991	55.3					
1992	41	1992	47.9					
1993	31.1	1993	29.7					

inghes

1 Inches

$$\frac{24.8}{32.9} \times 13.6 = 10.25$$

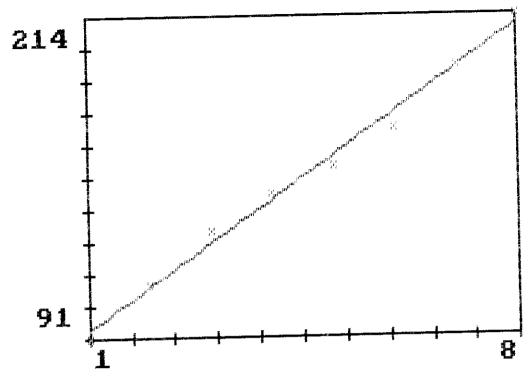
$$\frac{36.3}{32.9}$$
 x13.6 = 10.8

$$\frac{31.1}{32.5} \times 13.6 = 12.85$$

$$\frac{48.7}{27.1}$$
 x 13,6 =  $24.4$ 

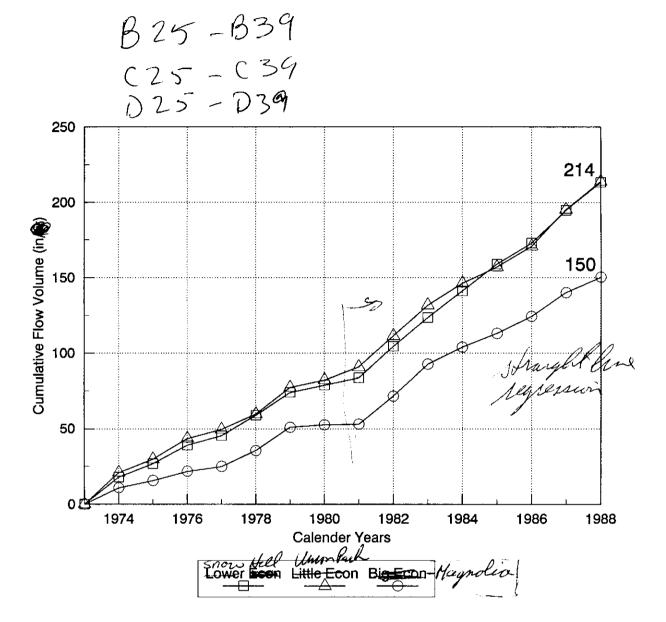
$$\frac{13.4}{27.1} \times 13.6 = 6.72$$

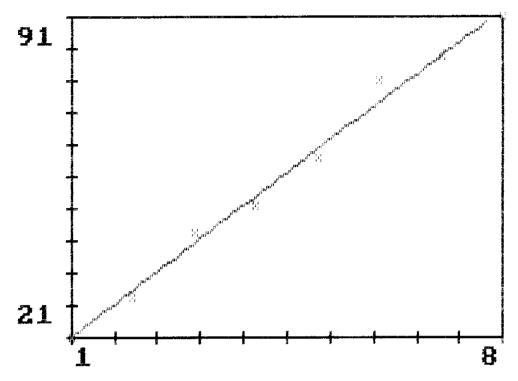
## LINEAR EQUATION UNION PARK



Y = 76.72144 + 16.7519 X R^2 = .9914158 PRESS PrtSc FOR HARD COPY OR (cr)

	90.9E
1 1	111.42
2 2	131.67
3 3.	146.15
4 4	156.99
5 5	170.69
6 7	195.1
9 9	213.90





Y = 10.23609 + 10.31976 X R^2 = .9900667 PRESS PrtSc FOR HARD COPY OR (cr) PRESS ANY KEY TO RESTART

NODE	Х	Y
1	1	20.75
2	2	29.57
3	3	43.36
4	4	49.47
5	5	59.75
6	6	77.29
7	7	82.29
8	8	? 90.92

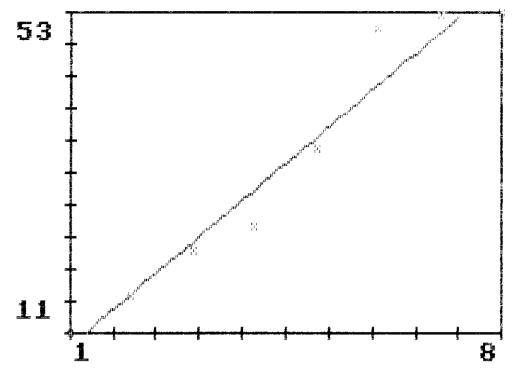
LINEAR EQUATION MAGNOLIA
RANCH

Y = 45.74497 + 13.42 X  $R^2 = .9845837$ PRESS PrtSc FOR UADD CADU AD /AXX

CURVE FIT

NODE	Х	¥
1 3 4 5 6 7 8	; 2 3. 4 5 6 7 8	52.91 71.65 92.75 104.02 118.11 124.35 140.01
co.	♥	

F9-RET TO CLEAR RE.



Y = 1.98286 + 6.894642 X R^2 = .9480612 PRESS PrtSc FOR HARD COPY OR (cr) PRESS ANY KEY TO RESTART

NODE	X	Y	
1	1	10.73	3
2	2	15.44	Ł
3	3	21.59	€
4	4	24.7€	5
5	5	35.2	
6	6	50.89	•
7	7	52.55	5
8	8	? 52.91	Ĺ

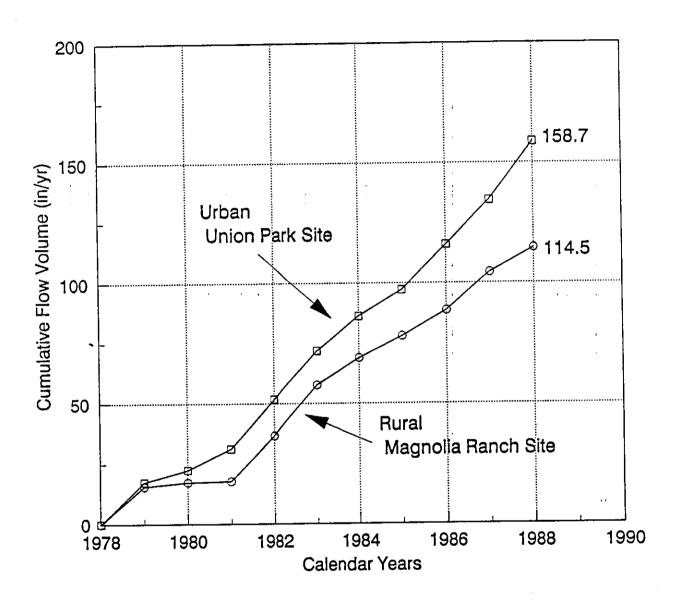
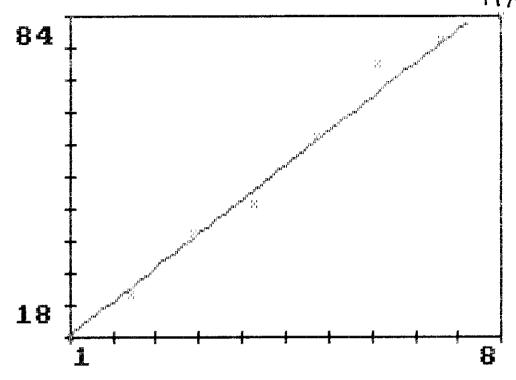


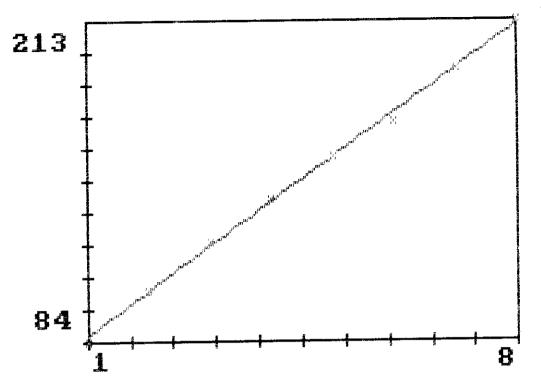
Figure 1. Cumulative Flow Volume Comparison / Econlockhatchee River

# LINEAR EQUATION SNOW HILL 1974-1981



Y = 7.891068 + 10.03226 X R^2 = .9843689 PRESS PrtSc FOR HARD COPY OR (cr) PRESS ANY KEY TO RESTART

NODE	Х	Y
1	1	17.70
2	2	26.65
3	3	38.93
4	4	45.3
5	5	58.97
6	6	74.1
7	7	78.9
8	8	83.74
_	2	



67.4557 + 18.13346 .9984549 PrtSc FOR HARD COPY ANY KEY TO RESTART 18.13346 X OR (cr)

NODE	Х	Υ
1	1.	<b>8</b> 8.74
(2)	<i>E</i>	104.51
.0	3.	123.56
4	<u>4</u> .	141.08
12	5	158.81
6	5	172,79
7	*7	194.6
8	έ	213.36
C3	ry	

#### ST. JOHNS RIVER BASIN ABOVE OKLAWAHA RIVER

#### 02233200 LITTLE ECONLOCKHATCHEE RIVER NEAR UNION PARK, FL

LOCATION. -- Lat 28°31'29", long 81°14'39", in SWk sec.32, T.22 S., R.31 E., Orange County, Hydrologic Unit 03080101, near right bank at Berry-Deese Road, 3,300 ft upstream from a tributary, 3 mi south of Union Park, 8.5 mi east of Orlando, and 13 mi upstream from mouth.

DRAINAGE AREA. -- 27.1 mi<sup>2</sup>.

PERIOD OF RECORD .-- October 1959 to current year.

GAGE. --Water-stage recorder. Datum of gage is 56.19 ft above National Geodetic Vertical Datum of 1929. Prior to Jan 12, 1960, and Oct. 21, 1972 to Nov. 14, 1983, nonrecording gage at same site and datum.

REMARKS . -- Records fair .

AVERAGE DISCHARGE. -- 29 years, 25.9 ft<sup>3</sup>/s, 12.98 in/yr.

EXTREMES FOR PERIOD OF RECORD. -- Maximum discharge, 1,640 ft<sup>3</sup>/s, Mar. 17, 1960, gage height, 11.64 ft; minimum discharge, 0.10 ft<sup>3</sup>/s, June 6,7, 1961; minimum gage height, 4.33 ft, May 14, 1967, May 6, 1971.

EXTREMES FOR CURRENT YEAR. -- Peak discharges greater than base discharge of 200 ft<sup>3</sup>/s (revised) and maximum (\*):

Date	Time	Discharge (ft <sup>3</sup> /s)	Gage height (ft)	Date	Time	Discharge (ft <sup>3</sup> /s)	Gage height (ft)
Nov. 4 Nov. 19 Nov. 27	0521 2400 0914	38£ 370 420	8.76 8.72 8,85	Mar. 19 Sept. 9	0923 0700	222 *450	8.27 *9.03

Minimum discharge, 7.7 ft<sup>3</sup>/s, May 11; gage height, 5.72 ft.

		DISCHARGE,	IN CUBIC	FEET	PER SECOND,	, WATER EAN VALU	YEAR OCTOBI ES	ER 1987	TO SEPTEMBER	1988		
DAY	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP
_	00	22	122	17	28	12	23	39	13	12	28	66
1	80		102	17	25	12	20	21	12	18	32	63
2	83	26		17	23	12	19	14	11	15	51	50
3 4	54	71	87		22	12	19	12	11	13	40	46
4	43	327	75	17		13	17	11	10	11	40	65
5	35	205	63	16	20	13	17	**	20			
6	29	144	53	16	19	52	15	10	34	22	36	205
7	25	112	48	15	21	40	14	9.5	36	18	40	225
á	23	91	44	15	22	28	13	8.9	53	14	38	202
9	20	77	41	16	25	23	13	8.6	28	12	4.5	393
10	19	84	39	23	22	46	12	8.1	21	13	38	256
		70	36	21	20	43	21	8,0	23	16	32	166
11	21	72		19	19	31	17	10	18	13	27	124
12	50	55	38		17	47	15	26	15	15	26	102
13	146	46	50	18		156	13	24	31	41	22	88
14	111	41	52	18	16		12	22	51	99	21	77
15	87	37	53	16	16	119	12	22	31			
16	72	34	56	16	33	98	12	15	28	46	23	69
17	58	44	46	15	25	85	11	12	21	29	28	61
18	47	193	39	15	21	78	11	11	18	23	35	54
19	41	199	33	14	19	196	11	10	16	20	100	48
20	39	293	30	14	18	158	12	9.5	16	20	69	46
20	33	235										
21	49	182	28	14	18	120	11	9.1	16	21	62	44
22	45	136	27	33	20	99	10	8.8	15	25	77	41
23	36	114	24	26	17	84	9.9	8.6	14	50	71	39
24	30	100	23	20	16	71	9.6	11	13	162	57	36
25	26	93	22	42	15	60	9.3	70	12	116	46	34
	24	97	21	130	14	51	9.3	33	11	67	45	37
26			20	74	14	46	8.9	32	12	48	76	64
27	22	371		50	13	39	8.5	22	15	37	124	44
28	21	260	20		13	34	8.3	18	16	32	88	37
29	19	189	19	40			8.7	15	14	27	85	33
30	17	148	17	35		29	0.7	14		24	70	
31	17		17	31		26		14		24	70	
TOTAL	1389	3863	1345	830	571	1920	393.5	531.1	604	1079	1572	2815
MEAN	44.8	129	43.4	26.8	19.7	61.9	13.1	17.1	20.1	34.8	50.7	93.8
MAX	145	371	122	130	33	196	23	70	53	162	124	393
		22	17	14	13	12	8.3	8.0	10	11	21	33
MIN	17	4.75	1.60	. 99	, 73	2.29	.48	.63	.74	1.28	1.87	3,46
CFSM IN.	1.65 1.91	4.73 5.30	1.85	1.14	.78	2.64	.54	.73	.83	1.48	2,16	3.86
T14.	1,51	2.00			• • •							

CAL YR 1987 TOTAL 17785.3 MEAN 48.7 MAX 744 MIN 4.7 CFSM 1.80 IN. 24.41 WTR YR 1988 TOTAL 16912.6 MEAN 46.2 MAX 393 MIN 8.0 CFSM 1.71 IN. 23.22

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PERIOD OF RECORD. --October 1959 to current year.

GAGE. --Water-stage recorder. Datum of gage is 56.19 ft above National Geodetic Vertical Datum of 1929. Prior to Jan 12, 1960, and Oct. 21, 1972 to Nov. 14, 1983, nonrecording gage at same site and datum.

REMARKS .-- Records fair.

AVERAGE DISCHARGE. -- 30 years, 25.8 ft 3/s, 12.93 in/yr.

EXTREMES FOR PERIOD OF RECORD. --Maximum discharge, 1,640 ft<sup>3</sup>/s, Mar. 17, 1960, gage height, 11.64 ft; minimum discharge, 0.10 ft<sup>3</sup>/s, June 6,7, 1961; minimum gage height, 4.83 ft, May 14, 1967, May 6, 1971.

EXTREMES FOR CURRENT YEAR. -- Peak discharges greater than base discharge of 200 ft 3/s and maximum (\*):

Date	Time	Discharge (ft <sup>3</sup> /s)	Gage height (ft)	Date	Time	Discharge (ft <sup>3</sup> /s)	Gage height (ft)
Nov. 23	1200	*625 401	*9.57 8.95	Sept. 2	0500	211	8.10

Minimum discharge, 3.7 ft<sup>3</sup>/s, May 9, June 5,6, gage height, 5.16 ft.

Day   OCT   NOV   DEC   JAN   FEB   MAR   APR   MAY   JUN   JUL   AUG   SEP			DISCHARGE,	IN CUBIC	FEET	PER SECOND,	WATER AN VAL	YEAR OCTOBER UES	1988	TO SEPTEMB	ER 1989		
1 31 13 38 16 17 42 19 8.6 8.7 13 6.3 10 12 155 33 34 14 43 17 42 19 8.6 8.7 13 6.3 10 12 155 12 71 47 82 93 17 40 18 8.6 8.1 15 15 12 71 15 15 17 17 17 18 18 18 18 18 18 18 18 18 18 18 18 18	DAY	OCT	ИОД	DEC	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP
2 20 14 43 17 42 119 8.6 9.4 5.1 15 12 71 14 78 29 39 17 40 18 8.4 8.1 4.4 11 15 47 15 17 5 57 27 36 16 36 18 8.4 7.5 3.9 8.0 12 34 16 40 21 30 16 31 15 9.4 5.8 20 11 9.0 59 8.8 31 18 28 16 29 17 9.3 5.0 77 9.6 10 38 19 28 16 26 16 8.4 4.7 55 7.9 11 37 10 30 16 25 15 24 15 8.1 4.6 26 8.6 15 33 11 28 16 25 15 24 15 8.1 4.6 26 6.6 15 33 11 28 12 26 16 49 17 9.3 5.0 77 9.6 10 38 12 28 16 25 15 24 15 8.1 4.6 26 6.6 15 33 11 28 12 26 16 49 17 9.3 5.0 77 9.6 10 38 12 28 16 25 15 24 15 8.1 4.6 26 8.6 15 33 11 28 12 26 16 27 15 20 14 7.7 5.3 14 26 38 24 15 12 26 16 16 71 15 20 14 7.7 5.3 14 26 38 24 15 12 25 15 21 13 35 14 17 12 11 20 9.1 17 24 24 16 22 13 31 14 19 12 6.7 4.6 10 27 34 25 15 15 21 13 35 14 17 12 11 20 9.1 17 24 24 16 22 13 31 14 17 12 11 20 9.1 17 24 24 16 24 15 27 13 16 12 9.3 5.8 8.7 16 14 33 19 20 19 13 22 13 16 11 8.1 5.1 13 37 14 38 19 19 19 14 24 13 16 11 8.1 5.1 13 37 14 38 19 19 19 14 24 13 16 11 8.1 5.1 13 37 14 38 19 19 19 14 24 18 24 19 12 26 7 4.7 4.7 11 56 18 13 16 22 17 14 17 21 18 8.1 17.7 4.7 11 56 18 31 16 14 33 19 19 19 14 24 18 18 13 22 18 15 11 7.7 4.7 11 56 18 31 16 22 17 14 17 12 11 6.9 9.6 9.9 9.9 25 19 23 23 23 17 417 21 228 14 11 6.9 9.6 9.9 9.9 25 19 23 23 24 18 85 19 79 13 10 5.6 4.1 18 9.9 15 46 38 29 14 17 7.7 4.7 11 56 18 31 26 18 28 21 13 19 9.9 5.3 3.9 7.8 14 34 34 33 35 24 18 15 11 7.7 4.7 11 56 18 31 26 27 15 15 86 19 79 13 10 5.6 4.1 8.9 15 46 38 39 30 13 62 18 57 9.9 13 10 5.6 4.1 8.9 15 46 38 39 30 13 62 18 57 9.9 13 10 5.6 4.1 8.9 15 46 38 39 30 13 62 18 57 9.9 13 10 5.6 4.1 8.9 15 46 38 39 30 13 62 18 57 9.9 13 10 5.6 4.1 8.9 15 46 38 39 30 13 62 18 57 9.9 13 10 5.6 4.1 8.9 15 46 38 39 30 13 62 18 57 9.9 13 10 5.6 4.1 8.9 15 46 38 39 30 13 62 18 57 9.9 13 10 5.6 4.1 8.9 15 48 39 15 48 39 15 48 39 15 48 39 15 48 39 15 48 39 15 48 39 15 48 39 15 48 39 15 48 39 15 48 39 15 48 39 15 48 39 15 48 39 15 48 39 15 48 39 15 48 39 15 48 39 15 5 48 39 15 5 48 39 15 5 48 39 15 5 48 39 15 5 48 39 15 5 48 39 15 5 48 39 15 5 48 39 15 5 48 39 15 5 48 39												12	155
5 57 27 36 16 36 18 6.4 7.3 3.8 5.6 2.5 1.5 1.7 1.7 1.2 1.1 2.0 9.1 1.7 24 24 1.3 1.6 2.2 1.3 2.1 1.4 1.7 1.2 1.1 2.0 9.1 1.7 2.4 2.4 1.6 2.2 1.3 2.9 1.4 1.7 1.2 1.1 2.0 9.1 1.7 2.4 2.4 1.8 1.9 1.9 1.4 2.4 1.3 1.6 1.1 1.7 1.2 1.1 2.6 3.8 1.7 1.6 1.8 1.3 1.9 1.9 1.3 1.9 1.9 1.3 1.5 1.9 1.9 1.3 1.5 1.9 1.3 1.5 1.9 1.5 1.9 1.3 1.5 1.9 1.5 1.9 1.3 1.5 1.9 1.5 1.9 1.3 1.5 1.5 1.5 1.5 1.5 1.5 1.5 1.5 1.5 1.5	2									5.1		12	
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TOTAL 801 1639 937 1476 645 398.5 250.6 222.7 424.2 549.4 729.9 1151 MEAN 25.8 54.6 30.2 47.6 23.0 12.9 8.35 7.18 14.1 17.7 23.5 38.4 MAX 78 417 71 270 49 19 16 20 77 56 92 155 MIN 13 13 18 13 13 9.7 5.3 3.9 3.9 5.9 9.0 20 CFSM .95 2.02 1.12 1.76 .85 .47 .31 .27 .52 .65 .87 1.42 CFSM .95 2.02 1.12 1.76 .85 .47 .31 .27 .52 .65 .87 1.42													
TOTAL 801 1639 937 1476 23.0 12.9 8.35 7.18 14.1 17.7 23.5 38.4 MEAN 25.8 54.6 30.2 47.6 23.0 12.9 8.35 7.18 14.1 17.7 23.5 38.4 MAX 78 417 71 270 49 19 16 20 77 56 92 155 MIN 13 13 18 13 13 9.7 5.3 3.9 3.9 5.9 9.0 20 CFSM .95 2.02 1.12 1.76 .85 .47 .31 .27 .52 .65 .87 1.42 CFSM .95 2.02 1.12 1.76 .85 .47 .31 .27 .52 .65 .87 1.42	31				53		9.7		11		12	20	
MEAN     25.8     54.6     30.2     47.6     23.0     12.9     8.35     7.18     14.1     17.7     23.5     38.4       MAX     78     417     71     270     49     19     16     20     77     56     92     155       MIN     13     13     18     13     13     9.7     5.3     3.9     3.9     5.9     9.0     20       CFSM     .95     2.02     1.12     1.76     .85     .47     .31     .27     .52     .65     .87     1.42       CFSM     .95     2.02     1.12     1.76     .85     .47     .31     .27     .52     .65     .87     1.00     1.58	TOTAL:	801	1639	937	1476			250.6	222.7				
MAX 78 417 71 270 49 19 16 20 77 56 92 155 MIN 13 13 18 13 13 9.7 5.3 3.9 3.9 5.9 9.0 20 CFSM .95 2.02 1.12 1.76 .85 .47 .31 .27 .52 .65 .87 1.42													
MIN 13 13 18 13 13 9.7 5.3 3.9 3.9 5.9 9.0 20 CFSM .95 2.02 1.12 1.76 .85 .47 .31 .27 .52 .65 .87 1.42					270								
CFSM .95 2.02 1.12 1.76 .85 .47 .31 .27 .52 .65 .87 1.42													
					1.76		. 47				.65		
	IN.			1.29	2.03	.89	. 55	. 34	.31	. ၁४	./3	1.00	1.30

CAL YR 1988 TOTAL 13692.6 MEAN 37.4 MAX 417 MIN 8.0 CFSM 1.38 IN. 18.80 WTR YR 1989 TOTAL 9224.3 MEAN 25.3 MAX 417 MIN 3.9 CFSM .93 IN. 12.66

#### ST. JOHNS RIVER BASIN ABOVE OKLAWAHA RIVER

#### 02233200 LITTLE ECONLOCKHATCHEE RIVER NEAR UNION PARK, FL

LOCATION.--Lat 28°31'29", long 81°14'39", in SWk sec.32, T.22 S., R.31 E., Orange County, Hydrologic Unit 03080101, near right bank at Berry-Deese Road, 3,300 ft upstream from a tributary, 3 mi south of Union Park, 8.5 mi east of Orlando, and 13 mi upstream from mouth.

DRAINAGE AREA. -- 27.1 mi<sup>2</sup>.

PERIOD OF RECORD. -- October 1959 to current year.

GAGE. --Water-stage recorder. Datum of gage is 56.19 ft above National Geodetic Vertical Datum of 1929. Prior to Jan 12, 1960, and Oct. 21, 1972 to Nov. 14, 1983, nonrecording gage at same site and datum.

REMARKS . -- Records fair.

AVERAGE DISCHARGE. -- 31 years, 25.6 ft 3/s, 12.83 in/yr.

EXTREMES FOR PERIOD OF RECORD. --Maximum discharge, 1,640 ft<sup>3</sup>/s, Mar. 17, 1960, gage height, 11.64 ft; minimum, 0.10 ft<sup>3</sup>/s, June 6,7, 1961; minimum gage height, 4.83 ft, May 14, 1967, May 6, 1971.

EXTREMES FOR CURRENT YEAR. -- Peak discharges greater than base discharge of 200 ft3/s and maximum (\*):

Date	Time	Discharge (ft <sup>3</sup> /s)	Gage height (ft)	Date	Time	Discharge (ft <sup>3</sup> /s)	Gage height (ft)
Feb. 23	2400	*123	*7.81				

Minimum discharge, 2.7 ft<sup>3</sup>/s, May 18,19, gage height, 5.07 ft.

		DISCHARGE	, IN CUBIC	FEET	PER SECOND, ME	WATER AN VAL	YEAR OCTOBER UES	1989	TO SEPTEM	BER 1990		
DAY	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP
1 2 3 4 5	25 24 22 20 19	11 11 11 10 9.8	8.0 7.7 7.6 6.8 6.7	29 28 26 25 25	13 14 14 13 12	29 26 24 22 21	29 18 14 12 10	7.2 6.5 6.4 5.6 6.7	7.0 11 7.7 20 15	9.2 8.6 8.8 11 10	12 11 12 12 10	12 10 10 9.4 8.8
6 7 8 9	18 19 53 44 39	9.4 9.4 9.1 9.0 9.0	6.7 7.3 8.3 33	26 27 27 27 26	12 12 12 12 12	20 19 19 19	10 10 10 10 10	6.1 4.9 4.9 4.8 4.7	17 73 35 26 21	9.7 9.0 9.2 7.9 7.7	9.0 8.4 7.6 7.6 11	8.1 7.6 7.5 7.2 6.4
11 12 13 14 15	35 32 32 29 24	8.8 8.4 8.3 8.2 7.9	31 30 35 32 27	25 25 24 24 23	24 24 18 17 16	16 14 13 12 12	18 21 15 12 11	4.7 4.2 3.8 3.7 4.1	19 21 16 12 11	6.9 5.9 27 22 28	54 35 22 25 24	6.2 6.1 6.0 5.9 5.7
16 17 18 19 20	21 19 18 16 15	12 13 9.8 8.9 8.6	23 22 32 36 32	23 22 22 22 20	15 16 16 14 13	11 11 11 10 9.9	10 9.4 8.9 10	3.3 3.0 2.7 2.7 8.4	9.3 9.6 23 15	65 34 26 22 19	20 36 26 22 23	5.8 7.4 6.7 6.2 5.6
21 22 23 24 25	14 13 12 12 11	8.1 8.1 9.0 11 9.9	54 42 77 70 60	19 19 17 16 16	13 13 56 96 57	9.1 8.7 8.2 7.8 7.6	8.8 8.1 15 24 16	6.7 4.7 6.7 5.2 4.3	9.9 8.6 10 14 12	17 14 13 12 12	19 19 42 32 23	5.1 4.9 4.7 4.4 3.9
26 27 28 29 30 31	11 13 13 13 12 12	9.9 10 10 10 8.7	52 46 41 37 33 31	15 14 13 13 13	47 38 33 	7.6 7.4 6.9 7.4 21 36	11 11 9.6 8.7 7.7	3.7 3.5 6.1 5.5 4.9 4.7	21 31 17 12 10	27 28 21 16 16	20 17 15 13 12	3.8 3.8 3.8 8.4 24
TOTAL MEAN MAX MIN CFSM IN.	660 21.3 53 11 .79 .91	287.3 9.58 13 7.9 .35	965.1 31.1 77 6.7 1.15	664 21.4 29 13 .79	652 23.3 96 12 .86 .89	463.6 15.0 36 6.9 .55	378.2 1: 12.6 29 7.7 .47	54.4 4.98 8.4 2.7 .18	525.1 17.5 73 7.0 .65	540.9 17.4 65 5.9 .64	612.6 19.8 54 7.6 .73 .84	215.4 7.18 24 3.8 .26 .30

CAL YR 1989 TOTAL 7759.7 MEAN 21.3 MAX 270 MIN 3.9 CFSM .78 IN. 10.65 WTR YR 1990 TOTAL 6118.6 MEAN 16.8 MAX 96 MIN 2.7 CFSM .62 IN. 8.40

#### ST. JOHNS RIVER BASIN ABOVE OKLAWAHA RIVER

#### 02233200 LITTLE ECONLOCKHATCHEE RIVER NEAR UNION PARK, FL

LOCATION.--Lat 28°31'29", long 81°14'39", in SWk sec.32, T.22 S., R.31 E., Orange County, Hydrologic Unit 03080101, near right bank at Berry-Deese Road, 3,300 ft upstream from a tributary, 3 mi south of Union Park, 8.5 mi east of Orlando, and 13 mi upstream from mouth.

DRAINAGE AREA, -- 27.1 mi<sup>2</sup>.

PERIOD OF RECORD, --October 1959 to current year.

GAGE. --Water-stage recorder. Datum of gage is 56.19 ft above National Geodetic Vertical Datum of 1929. Prior to Jan 12, 1960, and Oct. 21, 1972 to Nov. 14, 1983, nonrecording gage at same site and datum.

REMARKS.--Estimated daily discharges: Feb. 5-10, June 19-22, Sept. 3-28. Records fair except for periods of estimated daily discharges, which are poor.

AVERAGE DISCHARGE. -- 32 years, 26.4 ft 3/s, 13.23 in/yr.

EXTREMES FOR PERIOD OF RECORD. --Maximum discharge, 1,640 ft<sup>3</sup>/s, Mar. 17, 1960, gage height, 11.64 ft; minimum, 0.10 ft<sup>3</sup>/s, June 6,7, 1961; minimum gage height, 4.83 ft, May 14, 1967, May 6, 1971.

EXTREMES FOR CURRENT YEAR. -- Peak discharges greater than base discharge of 200 ft3/s and maximum (\*);

Date	Time	Discharge (ft <sup>3</sup> /s)	Gage height (ft)	Date	Time	Discharge (ft <sup>3</sup> /s)	Gage height (ft)
Apr. 6	2400	322	8.59	July 13	1800	*786	*9.89
Apr. 24	0200	551	9.22	July 30	0200	513	9.19
May 27	0600	314	8.57	Aug, 4	0600	204	8.28
May 31	0400	232	8.31	Aug. 16	0100	214	8.32
July 2	1500	244	8.35	_			-,

Minimum discharge, 3.3 ft<sup>3</sup>/s, all or part of each day, Jan. 13,14,15, gage height, 5.20 ft.

		DISCHARGE	, IN CUBI	C FEET	PER SECOND	, WATER EAN VALU	YEAR OCTOBER ES	1990	TO SEPTEMBER	1991		
DAY	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	JUL.	AUG	SEP
1 2 3 4 5	17 11 8.7 7.9 7.7	5.2 5.9 7.2 9.0 8.4	6.2 6.0 5.6 5.4 5.4	5.3 4.9 5.3 5.8 5.5	11 9.7 9.9 9.6 9.2	3.8 3.8 5.6 13 8.3	34 20 16 14 14	52 61 49 38 32	159 193 130 99 81	83 212 167 137 120	190 194 166 194 147	70 67 64 60 58
6 7 8 9 10	7.5 6.6 7.6 7.0 21	7.2 6.1 5.6 5.3 7.4	5.0 5.6 14 12 8.8	4.9 4.6 4.3 4.2 3.9	8.3 7.7 7.0 6.4 5.4	7.3 5.4 4.7 7.0 14	194 220 101 64 81	28 26 24 21 19	79 98 98 83 67	106 85 72 61 60	119 98 84 74 74	56 53 50 45 48
11 12 13 14 15	23 15 11 8.6 7.6	7.1 5.9 5.4 5.2 5.0	8.3 7.4 6.6 6.0 5.7	3.9 3.8 3.6 3.3 5.0	5.2 4.6 4.6 5.1	9.4 7.0 7.4 6.9 6.8	60 42 33 28 24	18 17 17 26 19	58 51 47 45 44	85 111 495 547 421	70 82 191 172 138	52 50 48 46 44
16 17 18 19 20	7.5 6.5 6.2 6.0 6.0	5.1 5.5 5.6 5.8 6.4	5.6 5.4 5.5 5.6 5.8	21 13 8.6 6.8 6.2	6.8 5.0 4.9 4.9 4.7	6.9 17 94 94 47	22 22 43 31 44	34 64 45 61 64	44 42 40 46 45	311 234 252 154 116	178 117 91 76 69	42 41 41 40 38
21 22 23 24 25	7.8 9.6 17 14 10	6.6 6.7 7.0 7.0 6.9	6.2 6.1 5.0 5.8 5.6	5.8 5.4 4.7 4.3 5.7	4.5 4.5 4.5 4.3 4.1	40 28 22 19 17	64 43 120 444 242	62 54 65 109 145	43 40 39 37 38	98 85 71 69 74	65 60 56 60 78	38 38 37 35 33
26 27 28 29 30 31	7.9 6.8 6.1 5.7 5.4 5.3	6.3 6.1 6.9 6.6 6.4	5.3 5.3 5.9 7.3 6.4 6.0	7.0 5.9 5.5 5.5 5.4 5.5	4.8 4.4 3.8	16 14 13 13 14 21	201 137 100 76 62	134 283 205 131 109 205	51 109 112 82 65	78 96 101 274 405 226	93 90 78 70 77 74	32 31 30 29 29
TOTAL MEAN MAX MIN CFSM IN.	295.0 9.52 23 5.3 .35	190.8 6.36 9.0 5.0 .23 .26	201.8 6.51 14 5.0 .24 .28	184.6 5.95 21 3.3 .22 .25	174.9 6.25 11 3.8 .23	586.3 18.9 94 3.8 .70 .80	86.5 7 444 14 3.19 2	217 1.5 283 17 1.64	72.2 193 37 2.66 6	406 174 547 60 . 43	3325 107 194 56 3.96 4.56	1345 44.8 70 29 1.65 1.85

CAL YR 1990 TOTAL 4893.8 MEAN 13.4 MAX 96 MIN 2.7 CFSM .49 IN. 6.72 WTR YR 1991 TOTAL 18687.4 MEAN 51.2 MAX 547 MIN 3.3 CFSM 1.89 IN. 25.65

#### ST. JOHNS RIVER BASIN ABOVE OCKLAWAHA RIVER

#### 02233200 LITTLE ECONLOCKEATCHEE RIVER NEAR UNION PARK, FL

LOCATION. -- Lat 28°31'29", long 81°14'39", in SWk sec.32, T.22 S., R.31 E., Orange County, Hydrologic Unit 03080101, near right bank at Berry-Deese Road, 3,300 ft upstream from a tributary, 3 mi south of Union Park, 8.5 mi east of Altamonte Springs, and 13 mi upstream from mouth.

DRAINAGE AREA. -- 27.1 mi<sup>2</sup>.

PERIOD OF RECORD. --October 1959 to current year.

GAGE. -- Water-stage recorder. Datum of gage is 56.19 ft above National Geodetic Vertical Datum of 1929. Prior to Jan 12, 1960, and Oct. 21, 1972 to Nov. 14, 1983, nonrecording gage at same site and datum.

REMARKS. -- Estimated daily discharges: Oct. 21 to Nov. 5, Nov. 13 to Jan. 22. Records fair except for periods of estimated daily discharges, which are poor.

		DISCHARG	SE, IN CUI	BIC FEET P	ER SECOND, DAILY	WATER MEAN V	YEAR OCTOBE ALUES	ER 1991 TO	SEPTEM	BER 1992		
DAY	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP
1 2 3 4 5	40 45 56 124 82	23 22 21 20 19	11 11 10 10	7.6 7.8 8.4 7.7 7.3	7.0 7.3 8.2 8.4 20	14 13 13 13 12	24 22 20 18 17	28 25 23 21 20	19 17 19 78 49	35 33 29 25 22	9.7 11 19 49 43	51 50 67 104 82
6 7 8 9	89 86 68 57 51	17 17 15 14 14	9.9 9.8 9.6 9.5 9.3	5.8 5.6 6.4 5.8 5.7	36 26 19 16 15	12 20 41 24 18	16 16 17 16 15	18 18 20 17 15	40 37 34 31 45	20 20 24 20 18	38 72 76 131 162	52 52 56 55 50
11 12 13 14 15	47 43 40 39 38	13 13 13 13 13	9.1 9.0 8.8 8.6 8.5	5.6 5.5 5.2 8.8 9.0	14 13 13 12 12	21 19 17 15 14	19 369 181 106 74	14 14 13 14	40 40 54 44 37	17 15 14 14 33	101 89 149 113 104	64 58 56 64 70
16 17 18 19 20	60 51 45 41 39	13 13 13 12 12	8.3 8.1 8.0 8.0 8.0	8.3 7.4 6.9 6.6 7.8	11 11 11 12 12	13 12 12 11 11	58 48 42 37 42	13 12 11 11 10	33 30 28 25 22	23 20 18 19 27	85 83 410 601 464	50 54 48 51 71
21 22 23 24 25	37 36 35 34 33	12 12 12 12 12	7.8 7.8 7.8 7.7 7.0	7.8 7.2 8.6 19	12 12 13 13	9.8 9.4 10 9.0 12	109 185 109 80 63	9.4 9.1 8.9 8.5 7.9	23 21 19 18 17	24 21 19 17 17	310 222 157 125 102	80 188 207 235 210
26 27 28 29 30 31	32 31 29 27 26 25	12 11 11 11 11	7.4 9.0 8.5 8.0 7.8 7.4	8.8 8.1 8.6 8.0 7.5	25 20 15 14	208 82 47 37 32 28	52 44 38 34 30	7.5 7.2 12 12 30 25	18 23 33 32 27	15 13 12 11 10 10	85 72 65 63 63 55	164 144 111 107 149
TOTAL MEAN MAX MIN CFSM IN.	1487 48.0 124 25 1.77 2.04	426 14,2 23 11 .52 .58	270.7 8.73 11 7.0 .32 .37	243.9 7.87 19 5.2 .29	424.9 14.7 36 7.0 .54	809.2 26.1 208 9.0 .96 1.11	1901 63.4 369 15 2.34 2.61	469.5 15.1 30 7.2 .56 .64	953 31.8 78 17 1.17 1.31	615 19.8 35 10 .73	4128.7 133 601 9.7 4.91 5.67	2820 94.0 235 48 3.47 3.87
STATIST	ICS OF M	ONTHLY ME.	AN DATA F	OR WATER Y	EARS 1960	- 1992,	BY WATER	YEAR (WY)				
MEAN MAX (WY) MIN (WY)	27.0 114 1970 3.16 1971	18.6 129 1988 2.67 1971	14.9 52.9 1970 2.22 1961	19.8 82.2 1986 2.73 1968	22.9 59.6 1983 3.58 1968	27.2 193 1960 3.51 1961	17.8 86.5 1991 1.64 1961	10.1 71.5 1991 .59 1961	23.8 137 1968 1.14 1962	38.5 174 1991 5.29 1980	50.0 133 1992 5.94 1980	57.8 171 1960 4.12 1970
SUMMARY	STATIST	ics	FOR	1991 CALEN	DAR YEAR	F	OR 1992 WA	TER YEAR		WATER Y	EARS 1960	- 1992
LOWEST HIGHEST LOWEST ANNUAL INSTANT INSTANT INSTANT ANNUAL ANNUAL 10 PERC 50 PERC	MEAN ANNUAL M DAILY ME SEVEN-DA ANEOUS P	EAN EAN AN Y MINIMUM EAK FLOW EAK STAGE OW FLOW CFSM) INCHES) EDS EDS		20183.5 55.3 547 3.3 3.9 2.04 27.71 130 34 5.4	Jul 14 Jan 14 Jan 8		•5.2			1570 .1 .2 1640 11.6	Mar 1	7 1960 7 1961 3 1961 7 1960 7 1960

e Estimated

#### ST. JOHNS RIVER ST. JOHNS RIVER BASIN ABOVE OCKLAWAHA RIVER

## 02233200 LITTLE ECONLOCKHATCHEE RIVER NEAR UNION PARK, FL

LOCATION.—Lat 28°31'29" long 81°14'39" in SW<sup>1</sup>/4 sec 32; T.22 S., R.31 E., Orange County, Hydrologic Unit 03080101; near right bank at Berry-Deese Road, 3,300 ft upstream from a tributary, 3 and south of Union Park; 8.5 mi cast of Altamonic Springs, and 13 mi upstream from mouth.

DRAINAGE AREA.—27.1 mi<sup>2</sup>.

CACK - Water-stage recorder. Damm of gage is 56.19 ft above sea level. Prior to Jan 12, 1960, and Oct. 21, 1972 to Nov. 14, 1983, nonrecording gage size and damm.

REMARKS. - Records fair.

e de la se		DISCHAR	GR. IN CUBI	C PEET PEE	SECOND,	WATER YEA	R OCTOBE	R 1992 TO S	EPTEMBER 1	993	ti i de	
	Arte de la constante de la con				MEA FEB	IN VALUES MAR	APR	MAY	JUN	JÜL	AUĞ	SEP
DÀY	OCT	NOV 32	DEC 33	JAN 17	47	27	51 50	14 14	20 16	19 18	18 <sup>3</sup> '16	79 72
1 2 3	122 98 249	31 30	31 29	16 16 16	.44 40 38	26 24 46	39 35	13 13	29 19 16	17 16 15	16 16 15	641 50 41
	432 315	30 30	277 26	17	36 33	36 31	⊹37: ⊹37 ∕	18 16	14	/15	14 13	41 113
6 7	195 230	32 30 28	25 24 24	16 16 17	33 33 32	28 27	31 35 35	14 13 12	12 11 10	15 11 13	12 11	359 199
8 9 10	205 153 120	28 27	23 30	-29 -25	29 32	25 23	28	12	9:5	26 44	11 12	130 106
11 12	132 153	26 26	33 28	24. 39.	31 12	19 16 89	23 19 17	11 11 10	14 39	36 29	11 10 10	123 93 74
13	120 97	26 25 23	25 23 23	32 29 55	35 30 28	77 59	16 15	9.7 9.1	19 28	29 36	19	62 50
#15   16	62 / 71	22	.23 22	116, 107	25 22	19	29 23	8.9 8.7	20 17 14	39 46 45	17 15 12 美	- 11 39
17 18 19	65 60 54	22 22 22	22 22	80 65	25 25 23	69 52 44	18 16 15	8.3 8.0 7.7	13 12	)7 30	11	35 32
20	19	24 26	22. 21	56 49	22	440	14 14	7.3 6.8	11 10	43 74	25 20	317 28 24
21 22 23	46 44 44	26 27	20 20	44 41 39	26 45 33	46 41 98	20 20	6.4 6.4	12 11 11	75 53 47	19 16 28	22 20
24 25	47 45	55 7 <b>8</b>	20 18	36	30	108 85	19 18	6.3 6.2	28	" 	47 32	23 38
26 27	42 40	.69 50 44	18 18 19	64 101 88	33 36 30	62 48	17 16 15	5.9 6.0 9.6	30/ 21 19	34 28 24	50 / W	29 24
28 29 30	38 37 34	39 36	19 18	71 60 52	===	40 35 31	15	31 33∦	19 	22 20	103 76	<u>-21</u>
31 TÓTAL	33 3452	986	17 723	1433	905 32.3	1454 46.9	737 24.6	366.3 11.8	543.3 18.1	996 32.1 75	739.9 23.9 103	2066 68.9 359
MEAN MAX	111 432 33	32.9 78 22	23.3 33 17	46.2 116 16	47 22	108 16	51 14 91	41 5.9 .44	49 8.8 67	13 1.19	9.9 .88	20 2.54 2.84
MIN CESM IN.	4.11 4.74	1.21 1.35	.86 .99	1.71 1.97	1.19 1.24	1.73 2.00	1 01	.50	675	11.37	1.02	
STATI	STICS OF !	MONTHLY M	ean data f	OR WATER	YEARS 19	60 - 1993	, BY WATE	R YEAR (W	Y)	38.3	49.2	58.1
MEAN	29.5 114	19.0 129	15.2 52.9	20.6 82.2	23.2 69.6	27.7 193	18.0 86.5 1991	10.1 71.5 1991	23.7 137 1968	174 1991	133 1992	171 1960 4,12
MAX (WY) MIN	1970 3.16	1980 2,67	1970 2.22 1961	1986 2,73 1968	1983 3.58 1968	1960 3.61 1961	1.64 1961	.69 1961	1.14 1962	5.29 1980	5.94 1980	1970
(NA)	1971	1971			endar yea		FOR 1993	NATER YE	iŘ.	WATER	YEARS 196	0 - 1993
9.7	ry Statis L-Total	TICS	POR	17526.	NG.		14401 39	.5		27.	4	·1960
ANNUA	L MEAN ST ANNUAL	MEAN	gen State	×47/.				a transfer a transfer		1570	Al Mar	1971 17 1960
LOWES	ST ANNUAL ST DAILY	MEAN FAN	eret 1811 - Erek Seit	601 5		19 13 7		9 May	27 22	7 - 11 K. M. J.	.10 Jun .20 Jun Mar	3 1961 3 1961
ANNU	AL SEVEN-	DEAK FLOI		5.	8 Jan	> <b>1</b>	488	Oct .04 Oct .7 May	4	1640 11	.64 : Mar .10 : Jun .02	17 1960 6,7 1961
INST	ANTANEOUS AT GUNOEF	PEAK STAI LOW FLOW (CFSM)		1	.77 .06		1 19	.46		1 13 61	.90	4 40 A
ANNU 10 P 50 P	AL RUNOFF ERCENT EX EBCENT EX	CEEDS CEEDS	w ·	109 24	<b>9</b> 4.		75 26 12	3"		14		
90 P	ERCENT EX	CEEDS		, /Þ	.9	2			-			

#### ST. JOHNS RIVER: ST. JOHNS RIVER BASIN ABOVE OCKLAWAHA RIVER

## 92233200 LITTLE ECONLOCKHATCHEE RIVER NEAR UNION PARK, FL

LOCATION—Lai 28°31'29", long 81°14'39", in SW<sup>1</sup>/4 sec. 32, T.22 S., R.31 E., Orange County, Hydrologic Unit 03080101, near right bank at Berry-Deese Road, 3.300 it upstream from a tributary, 3 not south of Union Park; 8.5 not east of Orlando, and 13 mill upstream from mooth.

DRAINAGE AREA -- 27.1 mil.

GAGE -- Water-stage recorder. Dutum of gage is 56.19 ft above sea level. Prior to Jan 12, 1960, and Oct. 21, 1972 to Nov. 14, 1983; nonrecording gage at size and datum.

REMARKS...-Récords fait.

n agrifian Lagran				C KUUT DER	SECOND, WA	TER YEA	R OCTOBE	R 1993 TO SI	EPTEMBER	1994		
		DISCHAR	ge, in cod		CE PHARMS	C THEORY	Treatment .	IAY	JUN	JUL	AUG	SEP
DAY	ост	NOV	DEC	JAN		MAR 27	APR 10	15	11	. 27	54 45	67 95
1	19 19 18 16	26 23 22 21	13 13 13 13	10 13 19 18	30 25 23 21 19	50 58 45 38	10 9.4 9.1 8.7	12 12 13 13	9.5 8.7 8,7	54 55 79 57	43 46 37	75 62 62
5 6 7	16 18 19 17 17	20 19 18 17 18	13 12 12 12 12 12	13 12 12 12	19 19 10 17	32 29 27 26	8.5 8.2 9.0 7.9 7.6	10 9.2 8.4 7.6 7.4	27 34 69 89 58	49 129 90 64 51	.66 197 154 137 187	52 45 42 73 73
9 10 11 12 13	21 19 17 15	18 17 16 16	12 12 11 11 11	11 11 11 12 32 21	16 15 22 08 40 36	28 24 22 20 19 18	7.4 7.1 7.7 7.0 7.5	7.2 6.7 6.4 6.5 6.2	42 51 41 38 40	42 35 30 30 46	138 127 135 158 149	67 59 117 208 151
15 16 17 18 19	16 34 27 23 20 18	16 15 15 16 16	11 13 11 10 10	17 15 44 39 28	30 28 55 69 62	17 16 15 15	7.3 6.6 6.1 6.1	5.7 6.6 9.5 7.0	41 162 355 172 114	40 33 29 38 30	144 112 99 138 104	129 188 125 143 234
20 21 22 23 24	17 20 58 46	15 14 15 15	11 11 11 11 11	24 21 19 18	53 46 42 42 43	14 14 13 13 13	11 43 23 17 14	7.3 6.0 5.4 5.1 4.9	91 73 65 56 46	34 36 28 24 35	62 69 67 72 101	123 98 85 175
25 26 27 28 29 30	34 31 28 25 23	15 15 15 14 13	14 12 11 12 11	17 17 17 16 19	32 32 29	12 12 11 12 14 12	15 17 15 13 13	4.7 4.5 4.4 8.9 25	39 35 30 28 28	40 57 72 76 63 65	146 140 118 90 71	331 219 160 130
31 TOTAL MEAN MAX MIN CPSM	731 23.6 58 14 .87	508 16.9 26 13 .62	363 11.7 15 10 .43	593 19-1 44 10 -71 -81	980 35.0 84 15 1.29 1.35	679 21.9 58 11 .81	341.2 11.4 43 6.1 .42 .47	267.6 8.63 25 4.4 .32 .37	1871.9 62.4 355 8.7 2.30 2.57	1540 49.7 129 24 1.83 2.11	3341 108 192 37 3.98 4.59	3992 133 435 42 4.91 5.48
IN.	1.00	MONTHLY ME	AN DATA		YEARS 1960	- 1994	, BY WATE	R YEAR (W	Y)		50.9	60 ¿2
HEAN HAX (NY) MIN	29.3 114 1970 3.16	19.0 129 1988 2.67	15.1 52.9 1970 2.22	20.5 82.2 1986 2.73 1968	23.5 69.6 1983 3.58 1968	27.6 193 1960 3.61 1961	17.9 86.5 1991 1.64 1961	10.1 71.5 1991 .69 1961	24.8 137 1968 1.14 1962	38.6 174 1991 5.29 1980	133 1992 5.94 1980	171 1960 4.12 1970
(MY)	1971	1971	1961 FOR		ENDAR YEAR		FOR 1994	WATER YE	<b>N</b> R	WATER	YEARS 196	0 - 1994
AMNUJ	RY STATIS LI TOTAL LI NEAN	eri, " Lista	-,	10842. 29.	5		15207. 41			28. 56. 7.	0	1960 1971
Lone: High Lone: Annu Inst Inst Annu Annu Inst	A TANADA I	Mean Hean Jay Mininu Peak Flow (CPSM) (CPSM) (CRSM) (CRSM)	E	359 5. 6. 1. 1.4 555 20	3 May 22		5. 548 9 4 1 20 113	.4 May .0 May Jun .21 Jun .2 May .54	29 22 17 17	1640 11 14 14 62 14	10 Jun 20 Jun 64 Ha .1 Jun .04	7 17 1960 7 1961 3 1961 1 3 1961 17 1960 17 1960 6:7 1961

#### ST. JOHNS RIVER BASIN ABOVE OKLAWAHA RIVER

#### 02233001 ECONLOCKHATCHEE RIVER AT MAGNOLIA RANCH, NEAR BITHLO, FL

LOCATION. --Lat 28°25'27", long 81°07'10", in SEt sec.4, T.24 S., R.32 E., Orange County, Hydrologic Unit 03080101, near center of span on downstream side of bridge on Wewahootee Road, 250 ft downstream from Disaton Canal, and 7 mi south of Bithlo.

DRAINAGE AREA. -- 32.9 mi<sup>2</sup>.

PERIOD OF RECORD. -- 1960, 1964-67 (one discharge measurement each year), October 1972 to current year.

GAGE .-- Water-stage recorder. Datum of gage is National Geodetic Vertical Datum of 1929.

REMARKS. -- Estimated daily discharges: Oct. 13-28. Records fair.

AVERAGE DISCHARGE.--16 years, 24.9 ft3/s, 10.28 in/yr.

EXTREMES FOR PERIOD OF RECORD.--Maximum discharge, 474 ft<sup>3</sup>/s June 21,22, 1982, gage height, 62.58 ft; maximum gage height 63.42 ft, Mar. 31, 1987; no flow for many days in most years; river dry at gage for many days in most years.

EXTREMES FOR CURRENT YEAR. -- Maximum discharge, 179 ft<sup>3</sup>/s, Nov. 4, gage height, 62.22 ft; no flow many days, river dry at gage many days.

		DISCHARGE,	IN CUBIC	FEET	PER SECO	ND, WATER MEAN VAL	R YEAR OCTO LUES	BER 1987	TO SEPTEMBER	1988		
DAY	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP
1	29	16	142	24	38	12	36	. 95	.19	. 25	.00	65
	45	19	132	23	36	11	32	1.5	.09	. 21	.03	64
3	41	45	123	22	34	10	29	1.5	.02	.18	. 54	55
2 3 4	34	150	114	22	32	9.7	26	1.1	.00	. 10	.49	46
š	29	172	106	21	30	9.7	24	. 69	.00	.02	.33	41
6	24	161	98	19	29	19	21	.42	.00	.07	1.6	41
7	20	156	91	18	29	24	18	.25	.04	.06	8.2	51
8	15	152	85	18	31	22	16	. 17	2.9	.01	4.7	63
9	13	144	80	19	35	20	14	. 13	7.2	.00	6.8	98
10	11	137	75	22	35	43	13	.10	7.6	.00	6.8	114
11	14	130	72	25	33	52	16	.06	5.2	.00	6.2	102
12	43	121	68	26	31	46	18	.03	3.2	.00	13	89
13	85	111	64	26	29	53	16	.01	1,8	.00	16	79
14	100	103	60	25	27	110	14	.01	1.1	.00	15	71
15	75	95	57	24	26	112	12	.01	9.3	. 33	13	64
16	52	88	59	23	26	98	10	.00	19	, 22	26	58
17	37	85	57	22	25	87	8.7	.00	13	. 12	118	51
18	31	127	52	21	24	83	7.2	.00	7.7	. 16	129	46
19	25	138	49	20	23	119	5.9	.00	4.7	. 11	106	41
20	23	146	46	19	21	124	4.8	.00	3.1	.03	88	37
20	20	140	40									
21	36	144	44	20	21	111	3.9	.00	2.6	.00	79	34
22	32	136	42	31	21	99	3.1	.00	3.2	.00	67	30
23	30	128	39	32	20	89	2.4	.00	3.5	.06	64	27
24	28	121	37	30	20	80	1.8	.00	3.0	. 50	97	24
25	26	115	36	42	19	73	1.3	.00	1.8	. 85	99	21
26	25	111	34	51	17	66	1.0	, 62	. 96	.38	86	19
27	23	154	32	50	16	59	.78	1.8	. 83	. 19	90	24
28	22	172	31	46	14	54	. 52	1.6	1.3	. 14	90	31
29	19	162	29	43	13	49	. 34	1.0	. 81	.08	83	27
30	17	151	27	41		4.5	.30	. 63	. 51	.04	76	23
31	15		25	40		40		. 36		.00	68	
TOTAL	1020	3690	2006	865	755	1829.4	357.04	12.94	104.65	4.11	1458.69	1536
MEAN	32.9	123	64.7	27.9	26.0	59.0	11.9	. 42	3,49	. 13	47.1	51.2
MAX	100	172	142	51	38	124	36	1.8	19	. 85	129	114
MIN	11	16	25	18	13	9.7	.30	.00	.00	.00	.00	19
CFSM	1.00	3.74	1.97	.85	.79	1.79	. 36	.01	.11	.00	1.43	1.56
IN.	1.15	4.17	2.27	.98	. 85	2.07	. 40	.01	. 12	.00	1.65	1.74
CAL YR WIR YR	1987 1988	TOTAL 13852.3 TOTAL 13638.8	7 MEAN 3			00. NIM 00. NIM	CFSM 1.15 CFSM 1.13	IN. 15.				

38.

#### ST. JOHNS RIVER BASIN ABOVE OKLAWAHA RIVER

#### 02233001 ECONLOCKHATCHEE RIVER AT MAGNOLIA RANCH, NEAR BITHLO, FL

LOCATION. --Lat 28°25'27", long 81°07'10", in SEk sec.4, T.24 S., R.32 E., Orange County, Hydrologic Unit 03080101, near center of span on downstream side of bridge on Wewahootse Road, 250 ft downstream from Disston Canal, and 7 mi south of Bithlo.

DRAINAGE AREA. -- 32.9 mi<sup>2</sup>.

PERIOD OF RECORD. -- 1960, 1964-67 (one discharge measurement each year), October 1972 to current year.

GAGE .-- Water-stage recorder. Datum of gage is National Geodetic Vertical Datum of 1929.

REMARKS. -- Estimated daily discharges: July 2-19. Records fair except for estimated daily discharges, which are

AVERAGE DISCHARGE.--17 years, 24.9 ft3/s, 10.28 in/yr.

EXTREMES FOR PERIOD OF RECORD. -- Maximum discharge, 474 ft<sup>3</sup>/s June 21,22, 1982, gage height, 52.58 ft; maximum gage height 63.42 ft, Mar. 31, 1987; no flow for many days in most years; river dry at gage for many days in most years.

EXTREMES FOR CURRENT YEAR. -- Maximum discharge, 217 ft<sup>3</sup>/s, Aug. 22, gage height, 62.52 ft; no flow many days; river dry at gage many days.

		DISCHARGE	, IN CUBI	C FEET	PER SECOND	), water Tean valu	YEAR OCTOBER JES	1988 T	O SEPTEMBER	1989		
DAY	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP
1	21	3.4	43	9.1	50	4.6	.48	.00	.00	. 16	5.6	124
-	19	3.0	39	8.5	46	4.9	.32	.00	.00	.29	8.4	116
2	30	2.7	34	8.1	41	6.7	. 22	.00	.00	.00	11	110
1 2 3 4	75	4.9	31	7.5	38	8.5	. 59	.00	.00	.00	26	104
5	75 86	8.0	28	6.8	35	8.2	3.5	.00	.00	2.5	33	99
Э	80	8.0	20	0.0	03	٠,ـ						
6	74	12	26	6.2	32	7.4	3.5	.00	.00	1.3	29	98
6 7	63	13	24	5.7	29	7.6	2.2	.00	.00	.74	22	94
8	55	12	23	5.3	27	10	1.3	.00	.00	. 46	17	89
9	48	11	21	4.9	24	11	. 83	.00	.00	.30	13	85
10	43	9.4	20	4,5	22	11	, 53	.00	.00	.18	13	80
10	43	3.4	20	-1.5								
11	38	8.3	20	4.3	20	10	.35	.00	.00	. 10	33	76
12	33	7.4	27	4.0	19	9.1	. 26	.00	.00	.06	53	72
13	29	6.7	29	3.7	18	8.3	. 19	.00	.00	. 12	65	68
14	25	6.1	28	3.5	17	7.4	.14	.00	.00	. 24	81	65
15	22	5.5	26	3.2	16	6.5	. 54	.00	.00	. 44	95	61
13	22	3.5	20	0.2								
16	20	5.0	24	3.0	15	5.7	1.5	.00	.00	1.1	103	57
17	18	4.6	23	2.8	14	5.0	1.1	.00	.00	2.2	106	52
18	16	4.3	21	2.6	13	4.2	. 73	.00	.00	4.0	113	49
19	15	3.9	19	2.3	12	3.6	.48	.00	.00	2.9	150	50
20	13	3.6	18	2.2	11	3,1	.32	.00	.00	2.2	169	50
20	13	3.5										, -
21	12	3.3	17	4.4	10	2.7	. 24	.00	.00	3.4	192	47
22	11	3.5	16	70	9,7	2.3	.20	.00	.00	4.0	212	43
23	9.5	60	16	106	9.0	2.1	. 14	.00	.00	3.9	210	41
24	8.4	88	15	98	7.8	2.2	.07	.00	.00	3.1	205	42
25	7.5	82	14	89	6.9	2.2	.01	.00	.00	2.4	202	43
				83	6.2	2.0	.00	.00	.00	1.5	199	45
26	6.4	72	13		5.5	1.7	.00	.00	.00	.98	187	44
27	5.6	64	12	78				.00	.00	.75	172	42
28	4.9	59	12	72	5.1	1.3	.00		.00	.40	159	39
29	4.3	53	11	67		. 98	.00	.00		22	147	37
30	3.8	48	10	61		. 80	.00	.00	.00			
31	3.6		9.8	56		. 63		.00		.73	135	
TOTAT	820.0	667.6	669.8	882.6	559.2	161.71	19.74	0.00	0.00 •	40.67	3166.0	2022
TOTAL			21.6	28.5	20.0	5.22	. 56	,00	.00	1.31	102	67.4
MEAN	26.5	22.3			20.0 50	11	3.5	.00	.00	4.0	212	124
MAX	86	88	43	106		.63	.00	.00	.00	.00	5.6	37
MIN	3.6	2.7	9.8	2.2	5.1			.00	.00	.04	3.10	2.05
CFSM	.80	. 68	. 66	.87	.61	. 16	.02		.00	.05	3.58	2.29
IN.	. 93	. 75	.76	1.00	. 63	.18	.02	.00	.00	.03	3.30	4.43

CAL YR 1988 TOTAL 9080.23 MEAN 24.8 MAX 129 MIN .00 CFSM .75 IN. 10.27 WTR YR 1989 TOTAL 9009.32 MEAN 24.7 MAX 212 MIN .00 CFSM .75 IN. 10.19

#### ST. JOHNS RIVER

#### ST. JOHNS RIVER BASIN ABOVE OKLAWAHA RIVER

#### 02233001 ECONLOCKHATCHEE RIVER AT MAGNOLIA RANCH, NEAR BITHLO, FL

LOCATION.--Lat 28°25'27", long 81°07'10", in SE% sec.4, T.24 S., R.32 E., Orange County, Hydrologic Unit 03080101, near center of span on downstream side of bridge on Wewahootee Road, 250 ft downstream from Disston Canal, and 7 mi south of Bithlo.

DRAINAGE AREA. -- 32.9 mi<sup>2</sup>.

PERIOD OF RECORD. -- 1960, 1964-67 (one discharge measurement each year), October 1972 to current year.

GAGE .-- Water-stage recorder. Datum of gage is National Geodetic Vertical Datum of 1929.

REMARKS. -- Estimated daily discharges: June 1-12. Records fair except for estimated daily discharges, which are poor.

AVERAGE DISCHARGE. -- 18 years, 24.2 ft3/s, 9.99 in/yr.

EXTREMES FOR PERIOD OF RECORD. -- Maximum discharge, 474 ft<sup>3</sup>/s, June 21,22, 1982, gage height, 62.58 ft; maximum gage height, 63.42 ft; Mar. 31, 1987; no flow for many days in most years; river dry at gage for many days in most years.

EXTREMES FOR CURRENT YEAR. -- Maximum discharge, 91 ft 3/s, Oct. 13,14, gage height, 61.24 ft; no flow for many days; river dry at gage for many days.

		DISCHAR	GE, IN CUI	BIC FEET	PER SECON	D, WATER MEAN VALI	YEAR OCTO	BER 1989 1	O SEPTEM	BER 1990		
DAY	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP
1	36	25	10	55	9.8	36	23	.32	. 20	.45	.00	.00
2 3	34	23 21	9.4 8.6	50	9.5	34	17	.18	.00	1.8	.00	.00
4	31 28	20	7.8	45 42	8.8	33	13	.09	.04	1.3	.00	.00
5	25	18	7.8	39	8.4 7.8	31 28	9.6 7.3	.05	.01	.70	.00	.00
						25	7.3	.00	.00	. 36	.00	.00
6 7	23	17	6.7	37	7.2	26	5.6	.00	.01	. 15	.00	.00
7	22	16	6.2	35	6.7	24	4.4	.00	.01	.05	.00	.00
8	28	15	7.0	33	6.3	22	3.4	.00	.90	.00	.00	.00
9	27	14	18	31	5.8	20	2.8	.00	. 50	.00	.00	.00
10	23	13	23	29	5.6	18	2.3	.00	.35	.00	.00	.00
11	21	12	22	28	10	16	9.6	.00	. 25	.00	.00	.00
12	27	11	20	26	14	15	11	.00	. 17	.00	.00	.00
13	80	11_	25	24	13	13	8.6	.00	.10	.00	.00	.00
14	88	9.7	26	22	13	12	6.2	.00	.01	.00	,00	.00
15	76	9.0	25	21	12	11	4.5	.00	.00	.08	.00	.00
16	66	12	23	20	11	9.8	3.4	.00	.00	, 57	.00	.00
17	59	14	21	19	10	8.8	2.6	.00	.00	. 33	.00	.00
18	54	14	26	18	9.1	7.8	2.0	.00	.00	. 14	.00	.00
19	51	13	33	17	8.2	6.7	2.3	,00	.00	.05	.00	.00
20	46	12	40	16	7.6	5.8	2.2	.00	.00	.00	.00	.00
21	41	11	52	15	7.1	5.1	1.6	.00	.00	.00	.00	.00
22	36	10	56	14	6.9	4.4	1.1	.00	.00	.00	.00	.00
23	33	10	77	13	20	3.7	1.9	.00	.00	.00	.00	.00
24 25	29	11	86	13	42	3.2	3.2	.00	.00	.00	.00	.00
23	26	11	84	12	47	2.7	2,5	.00	.00	.00	.00	.00
26	25	11	81	11	43	2.2	1.8	.00	.00	.00	.00	.00
27 28	28	11 12	77	11	39	1.9	1.2	.00	.00	.00	.00	.00
29 29	30 29	11	74 70	10	37	1.6	.89	.00	.00	.00	.00	.00
30	28	11	65	9.7		1.8	.69	.00	.00	.00	.00	.00
31	26	11	60	9.4 9.6		12	. 51	.00	.00	.00	.00	.00
				9.0		23		.00		.00	.00	
TOTAL	1176	408.7	1146.9	734.7	425.8	439.5	156.19	0.64	2.55	5.98	0.00	0.00
MEAN	37.9	13.6	37.0	23.7	15.2	14.2	5.21	.021	.085	. 19	.000	.000
MAX	88	25	86	55	47	36	23	.32	. 90	1.8	.00	.00
MIN	21	9.0	6.2	9.4	5.6	1.6	. 51	.00	.00	.00	.00	.00
CFSM	1.15	. 41	1.12	. 72	.46	. 43	. 16	.00	.00	.01	.00	.00
IN.	1,33	. 46	1.30	. 83	. 48	. 50	. 18	.00	.00	.01	.00	.00

.83 TOTAL 9583.52 MEAN 26.3 MAX 212 MIN .00 CFSM .80 IN. 10.84 TOTAL 4496.96 MEAN 12.3 MAX 88 MIN .00 CFSM .37 IN. 5.08 WTR YR 1990

#### ST. JOHNS RIVER BASIN ABOVE OKLAWAHA RIVER

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DRAINAGE AREA. -- 32.9 mi<sup>2</sup>.

PERIOD OF RECORD. -- 1960, 1964-67 (one discharge measurement each year), October 1972 to current year.

GAGE. -- Water-stage recorder. Datum of gage is National Geodetic Vertical Datum of 1929.

REMARKS. -- Records fair.

AVERAGE DISCHARGE.--19 years, 24.8 ft3/s, 10.24 in/yr.

EXTREMES FOR PERIOD OF RECORD. -- Maximum discharge, 474 ft<sup>3</sup>/s, June 21,22, 1982, gage height, 62.58 ft; maximum gage height, 63.42 ft, Mar. 31, 1987; no flow for many days in most years; river dry at gage for many days in most years.

EXTREMES FOR CURRENT YEAR. -- Peak discharges greater than base discharge of 179 ft 3/s and maximum (\*).

Date	Time	Discharge (ft <sup>3</sup> /s)	Gage height (ft)	Date	Time	Discharge (ft <sup>3</sup> /s)	Gage height (ft)
June 27	2200 2000	*296 291	*63.01 62.98	•			

No flow for many days; river dry at gage many days.

		DISCHARGE	, IN C	CUBIC FEET	PER SECONI	), WATER ŒAN VAL	YEAR OCTO	BER 1990	TO SEPTEMBER	1991		
DAY	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP
1 2 3 4 5	.00 .00 .00 .00	.00 .00 .00 .00	1.8 2.1 2.3 2.2 2.0	.31 .24 .20 .17 .15	.34 .44 .91 .87	.00 .00 .00 1.6 2.0	6.7 6.9 5.9 4.9 4.4	22 18 15 11 8.7	44 46 44 42 41	176 161 147 132 124	126 130 120 108 97	235 209 186 174 159
6 7 8 9 10	.00 .00 .00 .00	.00 .00 .00 .00	1.7 1.6 3.0 3.7 3.3	.12 .11 .15 .15	.52 .39 .30 .21 .16	1.9 1.8 1.6 1.8 2.2	7.2 8.5 8.0 6.3 4.8	6.4 4.6 3.5 2.7 2.2	44 45 44 40 36	136 136 127 120 122	90 84 75 70 68	141 124 111 105 97
11 12 13 14 15	.00 .00 .00 .00	.00 .00 .00 .00	2.7 2.3 2.1 1.9	.14 .13 .09 .04	.12 .09 .07 .06	2.1 1.8 1.5 1.3	3.6 2.8 2.1 1.6 1.2	1.7 1.2 .89 1.1 3.2	32 28 24 20 17	123 119 129 123 114	61 55 59 64 60	89 80 72 65 58
16 17 18 19 20	.00 .00 .00 .00	.00 .00 .00 .78 1.2	1.6 1.4 1.3 1.2	.69 1.1 1.0 .90 1.1	.00 .00 .00 .00	1.3 3.1 13 22 23	.93 .78 1.3 1.3 2.5	10 20 20 17 15	16 17 22 28 65	111 107 101 92 85	53 46 41 37 35	53 48 44 43 44
21 22 23 24 25	.00 .00 .00 .00	1.3 1.3 1.5 1.5	1.0 .92 .81 .67	.98 .80 .61 .49 .45	.00 .00 .00 .00	19 14 11 9.1 7.4	3.8 3.2 9.0 43 53	15 16 19 24 33	90 120 126 122 137	92 85 74 68 66	33 30 31 44 81	44 45 45 42 39
26 27 28 29 30 31	.00 .00 .00 .00	1.6 1.5 1.4 1.3	.35 .30 .42 .70 .56	.50 .52 .56 .53 .46 .37	.00	6.0 4.9 3.9 3.2 2.8 4.1	50 43 37 31 27	39 49 57 55 50 46	251 265 284 238 201	64 63 76 87 101 114	189 231 275 289 273 255	38 36 33 33 39
TOTAL MEAN MAX MIN CFSM IN,	0.00 .000 .00 .00 .00	16.28 .54 1.6 .00 .02	47.64 1.54 3.7 .30 .05	13.27 .43 1.1 .04 .01	5.21 .19 .91 .00 .01	168.60 5.44 23 .00 .17 .19	381.71 12.7 53 .78 .39 .43	587.19 18.9 57 .89 .58 .66	2529 84.3 284 16 2.56 2.86	3375 109 176 63 3.31 3.82	3210 104 289 30 3.15 3.63	2531 84.4 235 33 2.56 2.86

CAL YR 1990 TOTAL 1829.28 MEAN 5.01 MAX 55 MIN .00 CFSM .15 IN. 2.07 WTR YR 1991 TOTAL 12864.90 MEAN 35.2 MAX 289 MIN .00 CFSM 1.07 IN. 14.55

#### ST. JOHNS RIVER BASIN ABOVE OCKLAWAHA RIVER

#### 02233001 ECONLOCKHATCHEE RIVER AT MAGNOLIA RANCH, NEAR BITHLO, FL

LOCATION. --Lat 28°25'27", long 81°07'10", in SEk sec.4, T.24 S., R.32 E., Orange County, Bydrologic Unit 10N. --Lat 28°3030101, near center of span on downstream side of bridge on Wawahootee Road, 250 ft downstream from Disston 5 mi east of Canal, and 7 mi south of Bithlo. IAGE AREA . -- 27

DO OF RECORD.

.--Water-stag an 12, 1960,

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TOTAL MEAN MAX MIN CFSM IN.

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DRAINAGE AREA. -- 32.9 mi<sup>2</sup>.

PERIOD OF RECORD. -- 1960, 1964-67 (one discharge measurement each year), October 1972 to current year.

GAGE, -- Water-stage recorder. Datum of gage is National Geodetic Vertical Datum of 1929.

REMARKS	Recor	ds fair.										
		DISCHARGE	, IN CUBIC	FEET P	ER SECOND D.	, WATER AILY MEA	YEAR OCTOB N VALUES	ER 1991 T	O SEPTEM	BER 1992		
DAY	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP
1 2 3 4 5	44 51 63 74 80	35 33 31 29 27	7.4 7.3 7.1 7.4 7.5	3.0 3.3 3.7 3.5 3.1	1.4 1.2 1.0 .95 2.1	2.6 2.3 2.1 1.8 1.7	2.1 1.7 1.4 1.2 .94	9.1 7.4 6.0 4.7 4.6	9.6 6.2 6.7 8.3 8.4	108 115 107 92 77	3.6 5.1 19 26 28	76 75 72 72 73
6 7 8 9	94 115 126 133 140	26 24 23 21 20	7.2 7.1 6.7 6.3 6.0	2.8 2.6 2.3 2.2 2.1	3.7 3.6 3.1 2.6 2.4	1.6 3.3 5.8 5.4 4.1	.77 .65 .56 .49	4.2 3.5 3.1 2.5 2.0	7.4 7.4 7.4 7.7 8.4	65 60 60 54 50	68 152 171 168 185	67 60 54 54 56
11 12 13 14 15	138 129 119 109	19 18 17 15	5.6 5.3 5.2 5.2 4.9	2.0 1.9 1.6 2.3 2.8	2.3 2.3 2.2 2.2 2.0	3.9 3.8 3.5 3.1 2.7	.34 13 21 21 19	1.5 1.2 .94 .86 .86	9.2 12 15 22 25	44 38 32 28 24	216 233 288 284 267	62 111 117 129 136
16 17 18 19 20	94 86 77 70 64	14 14 13 13	4.4 3.9 3.7 3.5 3.2	2.4 2.1 1.8 1.7 2.2	1.9 1.8 1.7 1.6 1.4	2.3 1.9 1.6 1.4 1.2	16 14 12 10 9.7	.68 .56 .48 .36 .32	25 22 20 16 15	20 17 14 12 15	245 229 234 297 280	129 122 118 109 102
21 22 23 24 25	59 54 49 46 44	12 11 11 10 9.2	3.1 3.0 2.8 2.8 2.7	2.2 2.0 2.3 2.9 2.4	1.2 1.1 1.1 1.3 2.5	.96 .81 .81 .72	14 25 31 32 28	.24 .12 .02 .00	17 14 11 10 8.8	20 23 22 20 16	247 216 190 180 159	96 90 85 82 77
26 27 28 29 30 31	42 41 42 42 40 37	8.5 8.2 8.0 7.7 7.5	2.8 6.4 5.0 4.0 3.5 3.2	2.0 1.8 1.7 1.7 1.7	5.2 5.3 4.1 3.2	11 11 7.1 4.1 3.1 2.6	24 19 16 13	.00 .00 .00 .06 8.2	11 24 44 46 48	13 10 7.8 6.0 4.7 4.1	137 121 106 96 89 82	73 70 67 75 93
TOTAL MEAN MAX MIN CFSM IN.	2403 77.5 140 37 2.36 2.72	512.1 17.1 35 7.5 .52 .58	154.2 4.97 7.5 2.7 .15	71.7 2.31 3.7 1.6 .07	66.45 2.29 5.3 .95 .07	99.70 3.22 11 .72 .10	359.25 12.0 32 .34 .36 .41	78.50 2.53 15 .00 .08 .09	492.5 16.4 48 6.2 .50	1178.6 38.0 115 4.1 1.16 1.33	5021.7 162 297 3.6 4.92 5.68	2602 86.7 136 54 2.64 2.94
STATIST	ICS OF M	ONIHLY MEAN	DATA FOR	WATER Y	EARS 1973	·- 1992,	BY WATER	YEAR (WY)	)			
MEAN MAX (WY) MIN (WY)	34.5 142 1980 .000 1981	15.9 123 1988 .034 1981	13.1 64.7 1988 .12 1981	18.8 80.3 1986 .088 1985	17.2 80.9 1983 .027 1976	17.3 91.7 1983 .000 1975	18.9 91.7 1983 .000 1975	5.19 23.7 1982 .000 1975	18.4 181 1982 .000 1977	29.8 109 1991 .000 1981	46.8 162 1992 .000 1981	55.8 168 1979 .000 1980
SUMMARY	STATIST	ICS	FOR 19:	91 CALEN	DAR YEAR	F	OR 1992 W	TER YEAR		WATER 1	YEARS 1973 -	1992
ANNUAL TOTAL ANNUAL MEAN BIGHEST ANNUAL MEAN LOWEST ANNUAL MEAN LOWEST DAILY MEAN LOWEST DAILY MEAN ANNUAL SEVEN-DAY MINIMUM INSTANTANEOUS PEAK FLOW INSTANTANEOUS PEAK STAGE ANNUAL RUNOFF (CFSM) ANNUAL RUNOFF (INCHES) 10 PERCENT EXCEEDS 90 PERCENT EXCEEDS		Mean Ean Ean An Y Minimum Eak Flow Eak Stage CFSM) Inches) Eds Eds			Aug 29 Feb 16 Feb 16		13039.70 35.6 297 .00 .01 303 63.04 1.08 14.74 110 9.2 1.2	Aug 19 May 24- May 23 Aug 19 Aug 19	-28	24 49 471 474 63 10 71	5 Jun 21 00 Many 00 Many Jun 21 42 Mar 31 74	1983 1981 1982 days days 1982 1987

## ST. JOHNS RIVER ST. JOHNS RIVER BASIN ABOVE OCKLAWAHA RIVER

## 02233001 ECONLOCKHATCHEE BIVER AT MAGNOLIA BANCH; NEAR BITHLO, FL

LOCATION.-List 28°25'27", long 81°07'10", in SE'/s sec.4, T.24 S. R.32 E. Orange County, Hydrologic Unit 03080101, near center of span on downstream from Dission Canal, and 7 mi south of Bithlo.

DRAINAGE AREA. -32.9 mi<sup>2</sup>.

PERIOD OF RECORD.—1960, 1964-67 (one discharge measurement each year), October 1972 to current year.

GAGE.—Water-stage recorder. Dahum of gage is at sea level.

REMARKS.—Records fair.

Page 6		DISCHAR	GE, IN CUB	IC FEET PE	R SECOND, W. MEAN	ATER YE. VALUES	AR OCTOBE	R 1992 TO 5	SEPTEMBER	1993			
DAY	OCT	NOV	DEC	jan	FEB	MAR	APR 🚈	MAY	JUN	JUL	AUG	SEP - 18	
1 2	86 83	32 30	63 60	19 19 19	99 93 88	39 37 36	72 78 76	9.3 8.2 7.2	17 11 9.5	.79 .69 .49	.00 .00 .00	18 16 12	
3	114 140 143	27 26 24	58 55 52	20 21	83 76	40 41	72	6.4 5.6	7.4 5.3	.27 .46	.00	12 58	
. 6 7	140 171	23 21	49 46 44	21 21 21	74 70 66	41 39 37	68 65 61	4.0 4.0 3.2	3.6 2.4 1.8	.63 .37 .31	.00 .00 .00	83 131 153	
9 9 10	167 156 3146	20 19 12	41.7 43	26 29	62 60	35 33	57. 54	2.7 2.5	1.1	.43 .87 1.6	.00 .00	160	
11 12	149 156	17 16 16	45 44 41	33 42 43	58 59 57	30 26 60	50 46 42	2.0 1.6 1.3	.86 .68 .58	1.8 1.4 2.1	.00 .00 .00	184 186 179	
213 14 64-915	141 125 111	16 15	39 ≨ 38 ≟	44 49	53 49	72	/39 36	1.0 .73	.58 .65 .87	3.1% 2.8	.00	168 154	
16 12	100 93 86	14 13 13	36 34 33	67 82 85	45 43 43	72 69 67	38 38 36	.55 .38 .16 .07	2.6 1.7 .87	1.9 1.1 .57	.00 .00 .00	139 126 118	
19 19 20	80 73	13 13	31 29	85 85	43 41	65 63	34 31 29	.00	.38 .07	.24 3.4	.00	*109 99	
21 22 23	68 63 59	13 14 19	27 25 24	63 60 77	39 41 48	64 64 68 130	26 23 21	.00 .00 .00	.00 .00	6.9 4.4 2.6	.00 .00 .00	90 82 74	
24 25	56 53	47 67	23 22	73 70	45 41	107 94	19 17	.00	.00	1.4	.00	68 62	
25 27 28	50 47 43	70 69 69	21 20 20	82 102 109	41 42 // 41	86 78 72	15 14 12	.00	.32 .49 .73	.34 .07 .00	.00 .00	60 60 # 58	
29 30 31	40 37 35	68 66	20 20 19	110 107 103		66 61	10	8.3 17	.83 	.60 .60	11.01	_ <b>52</b>	
TOTAL MEAN	3011 97.1	988 29.6	1122 36.2	1827 58.9	1602 57.2 99	1868 60.3 130	1250 41.7 78	86[99 2.81 17	72.73 2.42 17	41.67 1.34 6.9	11.01 .36 11	2900 96,7 186	
MAX MIN CFSM	171 35 2:95	70 13 .90	63 19 1.10	110 19 1.79 2.07	39 1.74 1.81	28 1.83 2.11	10 1.27 1.41	.00 .09 .10	.00 .07 .08	.00 .04 .05	.00 .01 .01	2.94 2.94 3.28	
IN.	3.40	1.00	1.27		YEAR5-1973			YEAR (W	Y)				
MEAN	37.5	16.5 123	14.2 64.7	20.8 80.3	19.1 80.9	19.3 91.7	19.9 91.7	5.07 23.7	17.6 181 1982	28.4 109 1991	44.6 162 1992	57.7 168 1979	
MAX (WY) MIN	142 1980 2000 1981	1988 .034 1981	1988 .12 1981	1986 .088 1985	1983 .027 1976	1983 .000 1975	1983 .000 1975	1982 .000 1975	.000 1977	.000 1981	.000 1981	.000 1980	
(MX)	i i i				NDAR YEAR		for 1993 l	WATER YEA	R	WATER 1	(EARS 1973	- 1993	
SUMMARY STATISTICS ANNUAL TOTAL ANNUAL MEAN				14991,40 41,0			14680:40 40-2				25.1 49.5 1983		
HIGHE LOWES	ST ANNUAL C ANNUAL I ST DAILY	MÉAN MEAN MEAN		297	Aug 19		186	Sep 1		473	60 Jun	1981 21 1982 any days	
LOWES	T DAILY MI	EAN AY MINIMUM	i,		00 May 24 01 May 23		187	00 Many 00 Many Sep	days (3	474	00 M	ny days 21 1982 31 1987	
INSTANTANEOUS, PEAK FLOW INSTANTANEOUS, PEAK STAGE ANNUAL RUNOFF (CFSM) ANNUAL RUNOFF (INCHES)				1 24 16,95			62,22 Sep 13 1,22 16,60				63.42 Mar 31 1987 .76 .10.36 .73		
29 OL	RCENT EXC RCENT EXC	EEDS EEDS		119 16 1.			96 30	00		7.	5 00		

#### ST. JOHNS RIVER ST. JOHNS RIVER BASIN ABOVE OCKLAWAHA RIVER. 92233001 ECONLOCKHATCHEE RIVER AT MAGNOLIA RANCH, NEAR BITHLO, FL

LOCATION:—Let 28°25'27" long 81°07'10", in SE<sup>1</sup>/<sub>4</sub> sec 4, T24 S.; R. 32 E. Orange County, Hydrotogic Unit 03080101; near center of span on downstream from Dission Canal, and 7 m secule of Bithlo.

DRAINAGE AREA.—32.9 mi<sup>2</sup>.

PERIOD OF RECORD -- 1960, 1964-67 (one discharge measurement each year). October 1972 to current year.

GAGE.: Water-stage recorder. Datum of gage is at sea level.
REMARKS.—Records fair, except for period of estimated delty discharge, which is poor.

etrer Tibula Assalias		DISCHAI	RGE, IN CUB	IC PRET PER	SECOND, W	ATER YE	AR OCTOBER	1993 TO SI	EPTEMBER I	994		
			and the second		FEB	HAR	APR	MAY	JUN	יומנ	AUG	SEP
DAY	OCT 47 42 39 35	NOV 39 35 32 28 25	DEC 5.3 4.8 4.4 4.1 3.8	JAN : 1.4 1.9 3.5 8:9 12	18 46 42 39 36	60 65 75	5.1 4.2 2.8 1.9 •1.0	.36 .12 .00 .00	.00 .00 .00 .00 .00	47 45 44 42 40	24 22 20 19 19	56 51 466 42 38
5 6 7 8 9	31 29 27 25 26 27	23 21 19 18 17	3.4 3.2 2.9 2.6 2.5	10 9.6 8.9 8.1 7.3	34 31 29 26 22	73 69 63 59	e.70 e.50 e.34 e.20 e.10	.00 .00 .00 .00 .00	00 1.2 29 51 68	38 42 43 43 49 45	22 30 31 31 31 31	30 36 40 35
10 11 12 13 13 14	27 24 21 10 17	16 15 14 13	2.4 2.4 2.2 2.0 1.9	6.7 6.7 6.7 11	21 20 23 24 21	49 44 39 36 33	e.00 e.00 e.00 e.00	.00 .00 .00 .00	79 84 88 99 88	39 35 32 32 32 27	45 51 57 63	34 56 84 81
7 7 7 5 1 E	22 39 44 43	11 11 10 9.5 9.1	1.6 1.4 1.3 .97	11 9.9 20 30 28	21 20 46 78 93	30 24 21 20 18	.00 .00 .00 .00	.00 .00 .00 .00	84 100 110 99 94	20 18	* 1.62 E.33	98 97 92 95
20 21 22 23 24	42 41 40 45 49	8.6 8.1 7.6 7.2	.89 .99 1.0 .98	24 21 20 18 17	93 90 87 82 81	17 16 14 14 14	.00 .31 .67 2.5 3.1	.00 .00 .00 .00	94 96 98 92 84	19 19 18 16 16	54 53 53 50	153 167 172 183
25 26 27 28 29 30	56 60 59 56 51 45	7.3 7.6 7.6 7.1 6.5 5.0	2.2 2.7 1.8 1.7	15 14 14 12 14 39	76 70 63	12 11 8.8 7.6 6.9 6.0	2.2 1.6 1.3 .82 .50	.00 .00 .00 .00 .00	70 62	18 19 17 18 22 26	52 58 59 61 62 59	220 214 201 186 172
TOTAL HEAN HAX MIN CESM	1169 37.7 60 17	451.0 15.0 39 5.8 .46	1.5 70.83 2.28 5.3 .89 .07	421.6 13.6 39 1.4 .41	1362 48.6 93 20 1.48 1.54	1108.3 35.8 75 6.0 1.09 1.25	29.84 .99 5.1 .00 .03 .03	0.48 .015 .36 .00 .00	61,5 110 .00 1.87 2.09	915 29.5 49 16 90 1-03	1439 46.4 67 19 1.41 1.63	2955 98.5 220 30 2.99 3.34
10.	1.32	inoppity M	EAN DATA	FOR WATER	YEARS 197	3 - 199	I, BY WATER	YEAR (W	TY)	11372		59.6
MEAN MAX (MY) MIN	37.5 142 1980 -000	16.5 123 1988 .034	13.6 64.7 1988 .12	20.4 80.3 1986 .088 1985	20.5 80.9 1983 .027 1976	20.1 91.7 1983 .000 1975	91.7 1983 .000	4.84 23.7 1982 .000 1975	181 1982	.000 1981	44.7 162 1992 .000 1981	168 1979 .000 1980
(WY)	1981	See .	1981		ENDAR YEA!	R	POR 1994	WATER YE	ÁR	WATER	YEARS 197	3 - 1994
ANNUA	ry stati: L total L mean		.p.or	11350.	23		11767. 32.			25. 49.	5 60	1983 1981
HIGHE LOWES HIGHE LOWES ANNO INST! INST! ANNO ANNO 10 P	ST ANNUAL T ANNUAL ST DAILY T DAILY L SEVEN- WITAMEOUS WITAMEOUS	MEAN MEAN DAY MININ PEAK FLC PEAK STA (CPSH) (INCHES) CEEDS CEEDS	GE	12 81 17	.00 May 2 .00 May 2 .95	Ö	222 62 13 81 21	00 Hany 00 Hany Sep 49 Sep .98	days days 26	471 474 63 10 74	00 N 00 H Jul 42 Has 77	21 1982 lany days lany days 1 21 1982 31 1987

Estimated

## UNIVERSITY OF CENTRAL FLORIDA CIVIL AND ENVIRONMENTAL ENGINEERING

#### STORMWATER MANAGEMENT

by Marty Wanielista

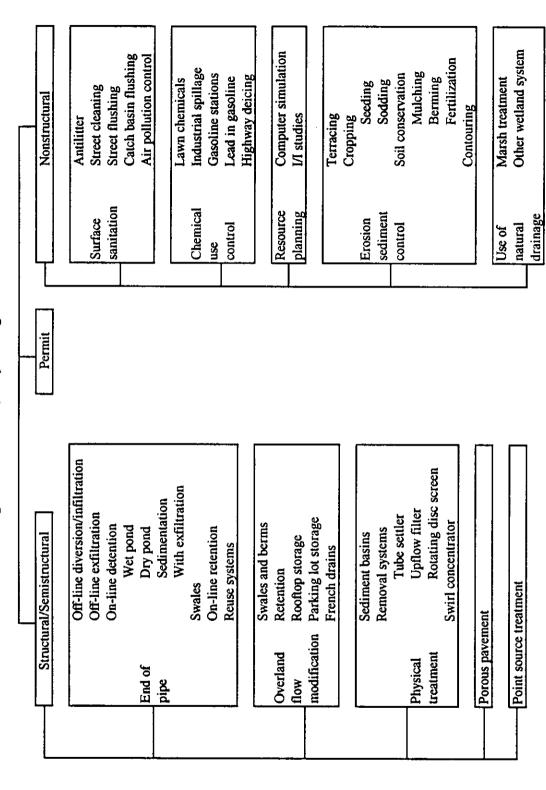
#### CLASSIFICATION OF TREATMENT SYSTEMS

There are three classifications for stormwater treatment systems as used for water quality improvement; namely, structural, nonstructural, and permit. The structural alternatives are ones that result in some physical alteration of the drainage, such as, detention ponds, ditches, and reuse systems. Nonstructural refers to ones that do not alter existing or natural drainage, such as, antilitter laws, and reduced chemical lawn care. The last and generally the most expensive is a permit to discharge. The permit usually results in large construction projects and is required when the public interest can not be served by other means. A listing of water quality management practices that improve water quality are shown in Figure 1.

#### STORMWATER MANAGEMENT IN THE STATE

Significant improvements in stormwater management practices have been developed in the State. Facts about the stormwater problem and management of it are attached in a publication by the author of this presentation.

Figure 1 Water Quality Management Practices



NONPOINT SOURCE / STORMWATER MANAGEMENT

## **\$EPA**

## **Facts About Stormwater** Management Programs in the State of Florida

### The Stormwater Problem



tormwater is that surface and ground water resulting from precipitation. In developed areas, surface water runoff is the major component of sewer and stream flows. The percentage of impervious areas that are directly connected to a sewer or other water conveyance system

determine the major portion of the volume and rate of discharge.

As an area becomes more urbanized, the peak rate of discharge, volume of discharge, and pollutant mass discharge increases. These effects are caused in part by modifications to surface drainage patterns, increased impervious areas (less infiltration and depression storage), and increased human and vehicle traffic. Human

activities add pesticides, fertilizers, animal wastes, oil, grease, solids, heavy metals, and other potential pollutants to the stormwaters.

The increased urbanization and many existing drainage practices in rural lands have caused flooding, erosion, and water quality degradation. In Florida, stormwater is the largest source of pollutants to lakes, rivers, and estuaries. In many lakes, it is the only major source of pollutants. On a statewide basis, stormwater as compared to regulated discharges (sewage and industrial treatment facilities) is the source of: (1) 80 to 95 percent of heavy metals; (2) 99 percent of all sediment; (3) 90 percent of oxygen demanding substances; and (4) 50 percent of the nutrients. Thus, severe environmental and economic impacts result when stormwaters are not managed.

## Stormwater Management Defined



tormwater management is a comprehensive, interdisciplinary body of knowledge required to design and operate stormwater programs to prevent flooding, reduce land loss due to erosion, maintain water quality, increase water availability, and provide funding

sources. A stormwater program must have goals and objectives that are implemented using a stormwater rule that specifies levels of performance. The minimum levels of performance in Florida are based on pre- versus post-peak discharge and 80 percent removal of pollutants. Stormwater management practices have been developed to meet these performance standards.

## **Stormwater Management Practices**



or a stormwater management practice to be successful, it must satisfy water quality and quantity considerations and have the necessary funding to be constructed and operated. There are at least five stormwater management practices that are now used in the State of Florida, namely (1) off-line retention by infiltration ponds and exfiltration trenches; (2) wet-detention ponds; (3) swales that both infiltrate and transport; (4) porous parking areas; and (5) alum injection.

## **Retention Using Infiltration Ponds**

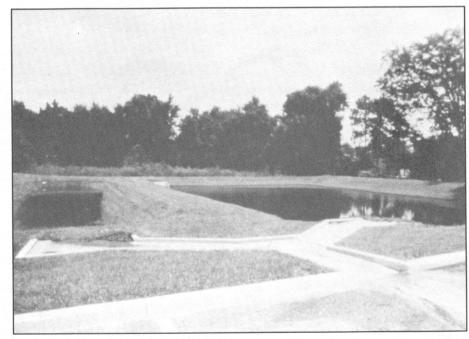
I

n infiltration pond is one that retains stormwater on-site in a surface pond. The soils beneath the pond must be capable of percolating the infiltrated water, and as such, the water table is usually below the bottom of the pond. The pond is designed to store a specific runoff

volume. This volume is determined from an analysis of storm events with

their rainfall volume (Wanielista, 1990). In the State of Florida, these rainfall analyses have defined the design criteria for pond volume as the runoff from the first inch of rainfall with a minimum of 1/2 inch over the watershed. The objective for sizing is to remove 80 percent of the runoff mass. The practice incorporates both

pollution control and ground water recharge; however, the impact of soluble pollutants on ground water must be carefully considered. The practice is used throughout the State where soils permit infiltration rates of at least 3-5 inches an hour. The infiltration pond is sized for the runoff from the first inch of rainfall; thus, additional runoff is diverted to direct surface water discharge or into a detention pond for peak discharge control.

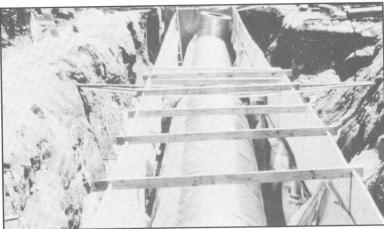


## **Retention Using Exfiltration Trenches**

W

hile infiltration ponds are constructed on the surface, exfiltration trenches are subsurface holding areas. They are also referred to as exfiltration pipes or pits. Highly permeable rock (i.e., limestone) or soils (sandy)

must be present. The most common construction practice is an excavation trench backfilled with coarse graded rock. Runoff is diverted to the exfiltration system. The system often includes perforated pipe surrounded by aggregate and a filter cloth. The pipe will increase the storage volume, since the rock aggregate has a porosity of about one-half or less that of the pipe. The design volume is calculated as the runoff from the first inch of rainfall, and a diversion structure or inlet control can be used to regulate runoff volumes greater than the design volume.



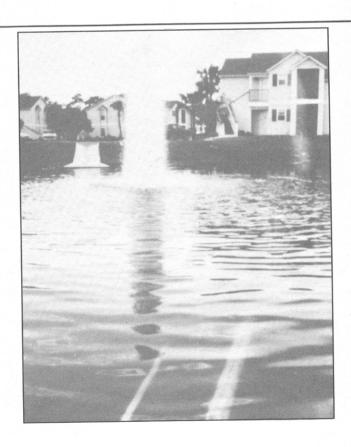
Exfiltration trenches with pipes, fabric wrap and rock are being used in central Florida. From these operating systems, it was concluded that the treatment volume should not be placed in the water table. In southern Florida, the pipe is frequently not used; however, the systems must be maintained to remove debris. Maintenance will vary directly with the amount of debris.

## **Wet Detention Ponds**



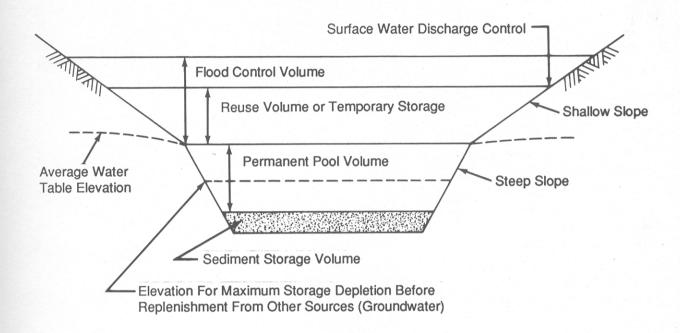
et detention ponds are excavated areas with a pool of water that exists throughout the year (permanent pool), a debris storage volume, and a temporary storage area. The ponds are used to attenuate (reduce) hydrograph peaks, pollutant loadings and concentrations of

pollutants, and to provide water for re-use purposes. Downstream water quality is improved because of sediment removal, plant uptake of nutrients, chemical transformation, and runoff water re-use. Temporary storage volume designs vary depending on the use of storage volume; however, the minimum size is calculated as one inch over the entire watershed. The total pond volume if used for peak attenuation is frequently greater than the temporary storage volume. A maximum depth for the permanent pool has been specified as six feet to minimize recycling of pollutants stored in the bottom muds. A vegetated area that leaves no more than 70 percent of the



permanent pool in open water is recommended. Short-circuiting of flow should be minimized.

These detention systems are found throughout the State in areas where the water table is of sufficient height to maintain the permanent pool. Recycling for irrigation purposes is being used more frequently to minimize the volume of discharge and pollutant loadings. In all cases, the invert elevation for the discharge structure should be above the seasonal high water elevation.



### **Swales**

S

wales are vegetated ditches that both infiltrate and transport runoff water. The top width to depth ratio must be equal to or exceed 6 to 1. Generally, the longitudinal slope is shallow to prevent erosion of the

ditch. The design infiltration volume is based on a State

rule that requires 80 percent of runoff from the three-year, one-hour design storm to be infiltrated over the length of the ditch (Livingston et al., 1988). Design equations are available and were developed by the Florida Department of Transportation (Wanielista, 1988).

Since long swale lengths are necessary to infiltrate runoff waters, swale blocks (berms) have been used to hold the runoff water until infiltration has occurred. The infiltration rate is critical, and care in selecting the rate is important.

Swales are used primarily along major highways within the right-of-way areas. However, some residential areas provide raised inlets to act as swale blocks and have been widely used in low ground water table areas.



## **Porous Parking Area**

A

pplications included both total and partial coverage using pervious concrete surfaces. They are used to reduce peak runoff and infiltrate

rainwater. Pervious concrete has a special formulated mixture of uniform open graded aggregate. Air entraining agents may be used. Proper installation is required. Partial coverage with a concrete grid having regularly interspersed void areas that are filled with sand, gravel, or sod is used in a few areas. Applications are found statewide and have been limited to parking and walkway areas.



## **Alum Injection**

B

uffered alum, which is a combination of aluminum sulfate and calcium compounds, is very effective for the reduction of phosphorus and some metals. The injection is being used in Tallahassee and Orlando.

### References

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NONPOINT SOURCE / STORMWATER MANAGEMENT

## **⊕EPA**

## **Urban Runoff Impacts to Receiving Waters**

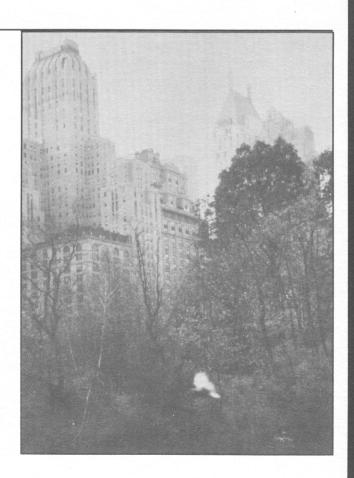
### Introduction

he EPA-sponsored National Urban Runoff Program (NURP) identified the potential of stormwater to adversely affect receiving waters and aquatic biota through increased frequency and duration of peak flow rates, erosion/sedimentation, eutrophication, or toxic impact. Assessments completed under state clean lakes and nonpoint source programs have identified the extent to which urban runoff is impairing water use.

### Stormwater Contaminants

he state assessments identified several categories of potential urban stormwater pollutants: suspended solids, nutrients, bacteria, oils/grease, toxic organics, and toxic inorganics (heavy metals). Critical pollutants were identified by: (1)

frequency of occurrence within the stormwater database, and (2) high concentrations relative to the EPA water quality criteria. An additional consideration was the degree to which urbanized stormflow hydrology alone impacted biota in natural stream courses. Potential impacts resulting from the presence of the above pollutants include: (1) physical impairment or habitat disruption to biota, (2) enrichment and subsequent eutrophication of receiving waters, and (3) exposure and physiological response to toxic substances by aquatic biota. The presence of such impacts are considered an impairment of the receiving water resource.





Automobile traffic is a major source of pollutants resulting from urban stormwater runoff.

### High Flows/Erosion/ Suspended Solids

Various urban runoff studies have effectively demonstrated the impacts of high flows, erosion, and deposition on urban streams and other sensitive receiving waters. Increased frequency and duration of high flows result in increased erosion-related impacts to (1) eroded sites, (2) conveyance systems including streams, and (3) sites of deposition. In-stream impacts are related to increased streambank erosion during high flows, increased turbidity and suspended solids concentrations, scouring habitat, and downstream depositional impacts that degrade habitat and reduce hydraulic channel capacities. Increased suspended solids concentrations and turbidity in streams can be detrimental to aquatic life (primary producers, benthic invertebrates, and fish) by interfering with photosynthesis, respiration, growth, and reproduction. The deposition of relatively fine-grained sediments in stream beds can dramatically reduce their value for insect

production and fish spawning. Erosion/ sedimentation impacts can be costly, requiring removal of deposited materials to restore water supply storage, flood control, habitat, and recreational benefits of impacted resources.

#### Nutrients

Increased nutrient (phosphorus, nitrogen) concentrations in stormwater have been shown to result in greater nutrient enrichment and associated algal productivity in lakes, embayments, and other quiescent receiving waters, often creating undesirable excessive growth conditions. Phosphorus is often emphasized as the nutrient controlling algal growth; phosphorus loading rates from urban areas have been determined to be three to seven times greater than undeveloped woodland. However, a preponderance of stormwater inflow has been demonstrated to inhibit algal growth as a result of the presence of toxic substances, despite elevated nutrient concentrations.

#### **Toxic Organics**

Toxic organic pollutants that are prevalent in urban runoff include pesticides, phenols, phthalates, and polynuclear aromatic hydrocarbons (PAHs). While some exceedances of EPA freshwater chronic water quality criteria have been reported, concentrations in general are sufficiently low to preclude significant impacts to aquatic biota. However, their potential for bioaccumulation and status as human carcinogens warrants continued consideration as pollutants of concern.

#### Toxic Metals

Toxic metals are the pollutants of greatest concern in urban runoff. Lead, zinc, copper, and cadmium have both a high frequency of occurrence and high absolute concentrations in stormwater; numerous exceedances of water quality criteria for these metals have been reported. Metals have the potential to bioaccumulate and persist in the environment.

EPA water quality criteria have been developed for both acute and chronic toxicity values from bioassays on representative biota; criteria are designed to protect 95 percent of aquatic species. Physiological effects of metals exposure include algal growth inhibition and zooplankton/fish mortality through gill adsorption and respiratory impairment.

## **Uncertainties of Toxic Impacts**



hile stormwater impacts related to aesthetics, hydrologic changes to stream habitat, elevated fecal coliform counts, and eutrophication have been adequately demonstrated, the adverse effects of toxicants have been more difficult to

establish. Although some water quality degradation may be occurring, such degradation has generally not been perceived to result in significant impairment to aquatic biota. For example, of over 10,000 fish kills investigated by EPA during the period 1970-1979, less than 150 were attributable to urban runoff. If present, potential toxic impacts have been more subtle and more easily overshadowed by larger, definite impacts associated with scour and sedimentation. Other sources of uncertainty regarding toxic pollutant impacts include the following:

 Water quality metals criteria and most stormwater analyses are based on total concentrations, whereas only the smaller dissolved fraction is directly related to toxicity. Criteria are therefore conservative as they also assume dissolution of the inert particulate fraction.

- Criteria are based on continuous bioassays for the defined exposure period. In reality, stormwater toxic exposure is intermittent and of short duration, whereupon receiving waters recover to relatively acceptable quality. These are recognized by EPA under the term "Estimated Effect Levels for Intermittent Exposure."
- Bioassays and water quality criteria are based upon end-of-pipe stormwater concentrations. Criteria therefore do not assume dilution capability by the receiving water.
- 4. Pollutant forms and concentrations are dynamic relative to product and use trends. For example, lead concentrations in stormwater have declined in recent years with the progressive conversion of the motor vehicle fleet to lead-free combustion engines. Similarly, some pesticide products are being retired in favor of new products being introduced to the market.

### **Summary Assessment of Urban Runoff Impacts**

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he majority of pollutant loading attributed to urban stormwater originates from endemic sources such as motor vehicle traffic and atmospheric fallout. While toxic metal criteria established from continuous exposure bioassays are

regularly exceeded in stormwater, receiving water resources and local perception often do not reflect

a corresponding beneficial use impairment of the resource. Possible reasons for this disparity are due to the conservative nature of water quality criteria designations and the complexity of biochemical cause and effect relationships. Instead, perceived or documented impairments focus on aesthetics from oil and floatable debris, species displacement from erosion/sedimentation in conveyance streams, and enhanced eutrophication potential from nutrient enrichment.



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